

359

„B“

11-89

11

PROJEKTNO PREDUZEĆE „ARHITEKT“ NOVI SAD

НСУТ
КУТИЈА 5
02-9

SAGLASAN SA PROJEKTOM INVESTITOR

**PROJEKTNO PREDUZEĆE
„ARHITEKT“
NOVI SAD
DIREKTOR**

ИМП. АРХ. ИМРЕ ФАРКАШ

PROJEKTNO PREDUZEĆE „ARRHITEKT“ NOVI SAD		ZNAK A	FAZA C ₂	DATUM II 1963	C 316
INVESTITOR Ž.T.P. N.Sad	PROJECTANT Ing. Imre Farkaš				
NAZIV OBJEKA, IMESTO GRADNJE Put željezničke pruge	SADRŽAJ Statički proračun -Krilo "B"-				

PROJEKTANT:

ING. IMRE FARKAŠ
Ovl. br. 4610/55 NRM

KONSTRUKTOR:

ING. D. BERISAVLJEVIĆ
Ovl. br. 68/55 NRM

SARADNICI:

- 1. TEHN. IVAN SABO**
- 2. TEHN. GUSTAV SILER**

SADRŽAJ:

1. STATIČKI RAČUN

TEHNIČKI IZVJEŠTAJ

Uz statički proračun Željezničke stanice Novi Sad (Hol ce Žekeonicama)

Konstruktivni sistem je zglobne celonjene naborane konstrukcije preko 8 polja u poprečnom pravcu (8 nabora), dok je u podušnom pravcu prosta greda se prepustom što je diktiralo funkcionalno rešenje razradjeno od glavnog projektanta stanice.

Rasponi su u polju 20,60 m, prepust 5,20 m a otvori nabora 6,00 m.

O konzolu naborane konstrukcije je obećena nastrešnica nad ulazom u hol.

Ova nastrešnica je delom obećena a delom se celanju na zidove.

Pri izvođenju ove prednje fasade обратити пажњу на sledeći redosled rada:

- a) Izbetonirati jednovremeno pos. H/19 (gredu s prepustom) kao delimičan nosač nastrešnice, pos H/20 (konsttinualnu redu) kao ležište za vešaljke, i ne skidati sa skele dok se ne izbetonira naborana konstrukcija pos H/l.
- b) Izbetonirati naboranu konstrukciju pos H/l i skiniti skelu.
- c) Kad se skela skinie ispod naborane konstrukcije pos H/l ubaciti vešaljke pos H/21, izvršiti njihovo ankerovanje i utezanje i injektiranje.
- d) Po izvršenom utezanju vešaljki pos H/21 oslobođiti skele pos H/19 i H/20.
- e) Namontirati nosače nastrešnice pos H/18 i H/18a.
Ovog redosleda radova se striktno pridržavati.

Ide je seksaonicama je obliks međuspratne konstrukcije "lvransenko" u proračunu dok je detaljom deta livena prekrsta konstrukcija, koja se prenosi preko sistema podvlača stubova na temelje. Raspon je 6,00 m.

Pojedini elementi radi težine iste dati su vodoravno-uglavljastog preseka (oplata ostaje ugradjena) isti nisu dati „17“ preseka radi oblaganja sa mermrom, jer isti se na rabiću slobodno nikako ne drži.

Izvodjač će pored nadzora od strane investitora obavezno tražiti prijem armature i od projektanta za sve delove konstrukcije. Bez prijema armature od strane projektanta zabranjuje se svako betoniranje.

Proračun naborane konstrukcije izvršen je po knjizi Sahnovski prevod i izdanje 1954 god.

Vetar je predst predst dela za Seksionicama na temelje.

Fundiranje je predviđeno samcima od nabijenog betona za $G = 2,0 \text{ kg/cm}^2$ prema podacima dobijenim od investitora.

Sve ostalo je izloženo sa najširim opisom u proračunu i detaljima.

Novi Sad, februara 1963 g.

In. Draško Berislavljević

Geometria de la casa

$$\text{Base} = 0.69 \quad \text{Alto} = 0.45$$

$$0.52 \quad \sqrt{0.69^2 + 0.45^2} = 0.76$$

a) Pila de piedra o piedra

$$a_1 = \frac{1}{2} \times 0.76 = 0.38 \text{ cm}$$

b) Pila de piedra o piedra

$$a_2 = \frac{2.5 + 1.5 + 0.9}{2} = 2.15 \text{ cm}$$

B) Pila de piedra o piedra de la casa (A)

$$\frac{1}{2} \times 0.76 = 0.38 \text{ cm}$$

c) Pila de piedra

$$a_3 = \frac{1.51}{0.16} = 9.44 \text{ cm}$$

Geometria de la puerta o puerta

Puerta A

a. Puerta o puerta

$$2 \times \frac{2.4 \times 1.4}{2} = 6.72 \text{ dm}^2$$

b. Puerta

$$2 \times 0.65 \times 1.4 = 1.9 \text{ dm}^2$$

c. Puerta o puerta

$$2 \times \frac{1.4 \times 0.1 - 0.07}{2} = 1.36 \text{ dm}^2$$

d. Puerta o puerta

a. Circular ducting

$$A_1 = 2.37 \text{ dm}^2, P_1 = 18.4 + \frac{14}{3} = 18.4 + 4.66 \text{ dm}$$

b. Ducting

$$A_2 = 12.53 \text{ dm}^2, P_2 = 18.4 + \frac{14}{2} = 18.4 + 7.0 \text{ dm}$$

c. Ducting or ducting

$$A_3 = 18.40 \text{ dm}^2, m = 2.5 = \frac{18.40 - 18.4}{18.40} = 4.9 \text{ dm}$$

$$P_3 = 18.40 + 4.9 = 18.40 + 12.0 \times 1.3 = 25.4 \text{ dm}$$

$$(P_3 = 0.25 \text{ m})$$

Minimum pressure

a. Circular ducting

$$P_0 = \Omega \times \frac{7}{100} \times \frac{A_1^3}{A_2^3}$$

$$= 2.09 \times 18.4 \times 1.3^3 = 50.7 \text{ dm}$$

b. Ducting

$$P_0 = \frac{27.0 \times 0.8^3}{12} \times 0.25 =$$

$$= \frac{27.0 \times 27.0^3}{12} \times 0.25 = 160.6 \text{ dm}$$

$$P_0 = 43.62 \times (80.6 - 2.1)^2 = 495.6 \text{ dm}$$

c. Ducting or ducting

$$P_0 = \frac{2}{3} \times \frac{7}{100} \times \frac{A_1^3}{A_2^3} + \Delta P_{\text{min}}^2 =$$

$$= \frac{2}{3} \times 18.40 \times 1.3^3 + 4.9^2 = 100.8 \text{ dm}$$

$$P_0 = 100.8 \times 1.01 = 101.8 \text{ dm}$$

Определение нормативов

$$W_0 = \frac{0,1745}{1,05 - 0,85} = 0,460 \text{ м}^3$$

$$W_0 = \frac{0,1745}{0,85} = 0,200 \text{ м}^3$$

$$A = \frac{40,00 \cdot 3,25}{2} = 64,00 \text{ м}^2$$

Факторы сопротивления

$$\phi = \frac{M_{12}}{M_{11}} + 0,76 + \frac{A_{12}}{A_{11}} + 6,66 \text{ м} \cdot \text{м}$$

A. СТАЛЬ НА ЧУМЕ

= 1,201 АСИД

$$\begin{aligned} &+ \text{Витиеван} 0,03 \\ &+ \text{Равн. щитов} 0,12 \\ &+ \text{СТАЛЬ ЧУМА} 0,03 \end{aligned} \times 0,66 = 1,201 \text{ м}$$

$$G = 0,243 \times 2,5 = 1,61 \text{ т/м}$$

$$G = 2,84 \text{ т/м}$$

Б. Консольная сталь

(ПЛАСТИЧНАЯ ЧУМА)

$$\frac{0,70 + 0,45 \times 0,12 \times 3,60 \times 2,50 \times 2}{2} = 1,50 \text{ т}$$

$$G = 1,30 \text{ т}$$

Б. Сталь до Рес. № 24

СОСТАВЛЕННО КОМПАНИЕЙ № 1

1. STAL 40 pos 4/21

2. STAL 40 pos 4/21

3. STAL 40 pos 4/21

4. STAL 40 pos 4/21

D. RODA DE VACUUM SISTEMA

6.00 Pos 4/21

3.00 + 4.00 = 2.42 t

LIT CANT

A. STAL 40 UA LUSMI

UA KONZOL

$$M_0 = -12.81 \times \frac{5.25^2}{2} + 1.20 + 5.01 = -45.20 \text{ t}$$

$$B_0 = +2.81 \times 5.25 + 1.20 = 16.50 \text{ t}$$

UA GOREDI

$$\text{siln } A_0 = -B_0 = -\frac{45.20}{20.5} = -2.20 \text{ t}$$

$$A_0 = 2.81 \times \frac{20.50}{2} - 2.20 = +27.00 \text{ t}$$

$$B_0 = -12.81 \times \frac{20.50}{2} + 2.20 = -31.40 \text{ t}$$

A = 27 t

B = 34.40 + 16.50 = 47 t

$$\text{siln } M = \frac{A^2}{2g} = \frac{27^2}{2 \times 2.81} = 129.0 \text{ t m}$$

— RODA TAKWA

$$L = \frac{2A}{g} = \frac{2 \times 27.0}{9.81} = 5.40 \text{ m} \approx \frac{9}{10} \text{ t}$$

3. STAL 40 Pos 4/21

UA KONZOL

100% - 100% = 0,0 t

Faktor 100% t

= 100% t

$$\text{Summe der } \Delta E = -\frac{9150}{20,50} = -4,45 \text{ t}$$

$$A = -4,45 \text{ t}$$

$$B = -4,45 \text{ t}$$

$$A = -6,45 \text{ t}$$

$$B = -4,45 + 6,45 = 2,00 \text{ t}$$

U Punkt 26 54202 Koenigswinter Bruecke 0,45 t

C, Sugg. da Ljuska

40% 0,007 dt

$$M_{B7} = 0,72 \times \frac{5,25^2}{2} = 10,20 \text{ dt}$$

$$F = 0,72 \times 5,25 = 3,80 \text{ t}$$

40% 0,007 dt

$$\text{Summe } \Delta E = A_5 + B_5 = \frac{8,00}{20,50} = 0,39 \text{ t}$$

$$A = 0,72 \times \frac{10,20}{2} = 4,90 \text{ t}$$

$$B = (0,72 \times \frac{10,20}{2} + 0,39) = 7,90 \text{ t}$$

$$A = 4,90 \text{ t}$$

$$B = 7,90 + 3,80 = 11,70 \text{ t}$$

$$\max M = \frac{6,90^2}{2 \times 0,72} = 31,64 \text{ dt}$$

$$\lambda = \frac{9 \times 6,90}{0,72} = 19,20 \text{ dt}$$

$$B_0 = 2,10 \text{ t}$$

$$B_A = 0,261 \text{ t}$$

$$\text{mit } \Delta_{10} = -0,5 \text{ t} \quad \Delta_{10} = \frac{15,40}{20,20} = 0,76 \text{ t}$$

$$\Delta_{10} = -0,20 \text{ t}$$

$$B_F = -0,20 \text{ t}$$

$$\Delta_F = -0,140 \text{ t}$$

$$B = 6,80 + 2,40 + 0,70 \text{ t}$$

U P O L Z U J E M E R C O D Z I E N T Y A N D O M E R C H A T

W K W P A J U T C H O D Z I E N T Y A N D O M E R C H A T

D Y U E U Z I O M E R C H A T

a, 2 a b o l y e r c h o d z i e n t y a n d o m e r c h a t

W o k o n z o l u

$$H_B = -(45,80 + 0,10 + 10,40) = -56,30 \text{ t}$$

$$B_A = -(10,10 + 17,40 + 2,40) = -30,00 \text{ t}$$

W o k o n z o l u

$$\text{mit } \Delta_{10} = \Delta_S = -B_S = -(2,00 + 6,40 + 0,50) = -8,90 \text{ t}$$

$$\Delta_{10} = 27,0 - 4,40 + 0,50 = 22,10 \text{ t}$$

$$B_F = -(31,40 + 4,40 + 7,90) = -43,70 \text{ t}$$

$$\Delta = 29,10 \text{ t}$$

$$B = 14,40 + 31,30 = 45,70 \text{ t}$$

$$F = 9,81 + 0,99 = 10,80 \text{ t}$$

$$A = 2 \cdot 870 = 16,90 \text{ m} \approx 0,23 \text{ t}$$

3.43

PREDSTAVLJENI KUĆIJI SIR. 33/1. MULTE TAKVA JE
DODATNOMA NE ZADNAKO PREDSTAVLJENI OTISKOM
TAKVA DA JE $C = 0,24$. PA JE MAX. VELIKOST
NEPODAVANJA DIMENZIJSKOGA, NE RODNOG
VADO JE LJEVINA OTISKOVIMA SAMO DOSEN. TOŽILO

$$\max u = 129,0 \text{ dm}$$

$$b_1 = \text{ZAPREMA } A + B + C + D$$

NA KONZOLI

$$MB = -(147,0 + 15,40) = -162,40 \text{ t}$$

$$Bd = 37,30 + 2,40 = + 40,20 \text{ t}$$

U A G. N. C. D.

$$\text{SPOREZNIK } A_S = -B_S = -(7,10 + 0,80) = -7,90 \text{ t}$$

$$\Delta d = +29,50 - 0,80 = +29,70 \text{ t}$$

$$Bl = -43,40 - 0,80 = -44,20 \text{ t}$$

$$A = +28,70 \text{ t}$$

$$B = +44,20 + 40,20 = +84,40 \text{ t}$$

$$q = 3,43 \text{ t}$$

$$\max u = \frac{2870}{2 \times 3,43} = 118,0 \text{ dm}$$

$$x = \frac{2870 \times 2}{3,43} = 16,30 \text{ m}$$

DIMENZIJSKA LJEVINA

HADONI

$$G_u = 162.6 \text{ N/mm}^2$$

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$$G_u = 162.6 \text{ N/mm}^2$$

$$G_u = \frac{162.6}{0.16} = 1016 \text{ N/mm}^2$$

$$G_u = \frac{1016}{0.16} = 6350 \text{ N/mm}^2$$

$$\text{SPEC. SU U GÖRÜŞÜ CA 444} \quad 20 \text{ MPa}$$

SAZI DÖRTŞÜTLÜ

$$\begin{aligned} \text{DA KESİZOLU} & \quad G_u = 79 \text{ kN/mm}^2 \\ \text{DA GRİD} & \quad G_u = 81 \text{ kN/mm}^2 \end{aligned} \quad \left\{ \text{G_u = 80 kN/mm}^2 \right.$$

ZATE-GÜTA SİLDİR EDİLATURASI

(ELC. UK UYLAŞ. OTAKAŞA)

DA KESİZOLU

$$T_c = M_c + 0.25$$

$$t = 0.112 \text{ m}$$

$$T_{max} = \frac{D_{max}}{2}$$

$$D_{max} = \frac{785 \times 2 \times 0.112 - 85}{2} = 90.4 \text{ t}$$

$$T_{ax} = \frac{96}{4.4 \times 1.4} = 16.9 \text{ kN}$$

$$\text{UYLAŞMA : } T_{ax} = G_u \cdot 0.25$$

DA GRİD :

$$T_c = f \cdot M_c = 1.03 \cdot 0.112 = 1.14 \text{ t}$$

$$t = 0.112 \text{ m}, \quad G_u = 2 \text{ t/mm}^2$$

$$T_{max} = \frac{D_{max}}{2}$$

$\Delta_{\text{ext}} = 125 \times 10^{-6} \text{ cm}^2$

$\approx 10 \text{ cm}$

$\approx 100 \cdot 30 \cdot 10^{-6} \text{ cm}^2 \cdot 0.001$

Antwurde Δ_{ext} mit $\Delta_{\text{ext}} = 10^{-6} \text{ cm}^2$

$\approx 40 \%$ (10 cm^2) $\approx 20 \text{ mm} \cdot 0.001$

40% (10 cm^2) $\approx 0.12 \text{ cm}^2 \text{ statt } 0.001$

Gesuchte Wärmeleitfähigkeit
1000 W/mK bei 20°C

Durchmesser

Radius $r = 5 \text{ cm}$

$T_b = 67.3 \text{ K cm}^2$

Wärmeleistung

10 Watt

$T_a = 5.0 \text{ K cm}^2$

$\approx T_a = 72.3 \text{ K cm}^2$

($\approx T_a = 0.72 \text{ K cm}^2$)

Statische Formel

Für obiges Volumen $L = 10 \text{ cm}$ bei 20°C

$S = 4.8 \times 7.5 + 3.0 \cdot 5.7 = 51.1 \text{ cm}^2$

Wärmeleistung $Q = 20 \text{ W}$

$$Q = \frac{\Delta T \cdot A}{L} \Rightarrow 20 = \frac{22 \cdot 51.1}{10} \Rightarrow 0.76 \approx 0.7 \text{ cm}$$

Gegebene Länge $L = 10 \text{ cm}$

$$M_e = T_a - T_b = 7.7 \text{ K}$$

Material: Metall

\rightarrow Wärmeleistung $Q = 174.5 \text{ cm}^2$

Wärmeleitung

Wärmeleiterfläche

$$60.3 \times 0.82$$

$$42 \text{ cm}^2$$

$$11.07 \times 17 = 191.9 \text{ ft}^2 + 61.0 \text{ cu}$$

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$$(11.07 \times 17) \times 0.792 = 191.9 \text{ cu}$$

$$11.07 \times 17 = 191.9 \text{ ft}^2 + 61.0 \text{ cu}$$

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$$\begin{array}{cccccc}
 & & \frac{e}{2} & \frac{e}{4} & \frac{3e}{8} & \frac{e}{2} \\
 G_0 = & 0 & -37 & -48 & -70 & -73 \\
 G_u = & 0 & +23 & +39 & +47 & +49
 \end{array}$$

TRANSVERSALNA SILE
 - HERODANHO OPT. POD A+B+C+D
 - OSLOHAC DODA PREDSTAVI

$$\begin{aligned}
 x = 0 & Q = Be = 44,20 \text{ t} \\
 x = \frac{e}{2} & Q = 44,20 - 3,53 \times 2,57 = 39,10 \text{ t} \\
 x = \frac{e}{4} & Q = 44,20 - 3,53 \times 5,13 = 26,00 \text{ t} \\
 x = \frac{3e}{8} & Q = 44,20 - 3,53 \times 7,70 = 16,90 \text{ t} \\
 x = \frac{l}{2} & Q = 44,20 - 3,53 \times 10,25 = 7,40 \text{ t}
 \end{aligned}$$

TANGENCIJALNI DAPONI U NEUTR. LINII

* STATICKI MOMENAT

GORENE OZNAČE

$$T_1 = 2,79 \text{ dm}^3$$

$$M_1 = (1,45 - 0,77) \cdot 0,05 = 0,33 \text{ m}$$

UZBOZOR

$$T_2 = 2 \times 1,43(18,1 - 7,7) = 29,60 \text{ dm}^3$$

$$\frac{1,31 - 0,77}{2} = 0,22 \text{ m}$$

$$S = 2,79 \times 11,3 + 29,6 \times 5,2 = 185 \text{ dm}^2$$

$$\alpha = \frac{Q}{2h} + \frac{S}{J}$$

$$\alpha = \frac{Q}{2 \times 0,05} \times \frac{0,085}{0,2073} = 550,6$$

Σ = 2400 : 19,0 = 140,0 Σ = 1600 : 12 = 133,3

Σ = 2400 : 19,0 = 140,0 Σ = 1600 : 12 = 133,3

Pogodno je uporabiti rednički podstavak na 12 cm.

A delu DD x = 0 do x = $\frac{l}{4}$ = 4,0 cm

A delu DD x = $\frac{l}{4}$ do x = $\frac{3l}{4}$

Dopolnitivo, kada je su?

$x = 0 \text{ do } \frac{l}{4} = 4,0 \text{ cm } \Sigma = 2,0 / 8 = 1 / 2$

Σ = 1600 : 16,0 = 100,0 Σ = 1600 : 4,0 = 400

PODUŽNA AQUATURA U DODECIJIM PROSEČIMA

12 POKOSA KONSTRUKTIVNE SLEDE:

$$F_a = I_s : \frac{H}{M_s}$$

$$x = \frac{l}{2}$$

$$F_a = 80 \text{ cm}^2$$

$$x = \frac{3l}{8}$$

$$F_a = 80 \times \frac{105,0}{129,0} = 77 \text{ cm}^2$$

$$x = \frac{l}{4}$$

$$F_a = 80 \times \frac{103}{129} = 64 \text{ cm}^2$$

$$x = \frac{l}{8}$$

$$F_a = 80 \times \frac{61}{129} = 38 \text{ cm}^2$$

$$x = 0 \quad F_a = 38 \text{ cm}^2 \text{ (konstruktivno)}$$

POTREBNA ZATEZANJA AQUATURA PO VREDNOSTI SILA DODECIJADA

DIZAJNIRAN SKICUĆ UNIČOMA I U CENTRALNOJ OBLOZI JE POENI SKICI

$$0 = 4,60$$

$$2,37 \quad 0,37 \quad 3,00$$

на сюжет 6/15 см россия тишина

веса

шарыл #

на дну сюжет $R = 10$ см

$$\tilde{C}_m = \frac{6,7 \times 0,23 + 1,6 \times \sqrt{2}}{0,92} = 3,4 \text{ кг/см}^2$$

на дну сюжет $R = 10$ см

$$\tilde{C}_m = \frac{6,7 \times 0,23 + 1,6 \times \sqrt{2}}{0,92} = 4,3 \text{ кг/см}^2$$

на дну сюжет $R = 8$ см

$$\tilde{C}_m = \frac{6,7 \times 0,23 + 1,6 \times \sqrt{2}}{0,92} = 5,1 \text{ кг/см}^2$$

$$\alpha = \frac{10,0 - 1,4}{10,0} \times 5,14 = 4,60 \text{ кг}$$

затяжка сюжет

$$Z_1 = \frac{12,5 \cdot 10,0}{2} \times 2,37 \times 12 \times \frac{1}{\sqrt{2}} = 21200 \text{ кг}$$

$$T \times S = \frac{21200}{1400} = 18,6 \text{ см}^2$$

$\phi 12/15$ см сюжет $= 9,40 \text{ см}^2$

$$Z_2 = \frac{10,9 \cdot 6,9}{2} \times 2,37 \times 10 \times \frac{1}{\sqrt{2}} = 10600 \text{ кг}$$

1. Vrh - 10 cm, 2. Vrh - 10 cm

Streljivo: $\varnothing = 7.30 \text{ cm}^2$

$$Z_1 = \frac{3.9 \cdot 0}{2}, 460 \times 2 \times \frac{\pi}{32} = 11600 \text{ kg}$$

$$fus = \frac{11600}{4600} = 7.30 \text{ cm}^2$$

$\varnothing = 8.35 \text{ cm}$, $\varnothing = 7.30 \text{ cm}^2$

Na desnoj strani je učinjeni izmjeri
na kojima je vidljivo da je
transverzalne silove pređene

Na osloncu $B_d = 40,00 \text{ t}$

Na kracu $Q_u = 21,60 \text{ t}$

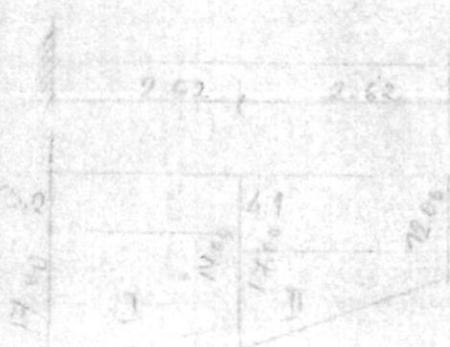
$$\bar{C}_{B_d} = \frac{40,00 \times 0,185}{2 \times 0,08 \times 0,2078} = 22,0 \text{ kg/cm}^2$$

$$\bar{C}_{Q_u} = \frac{21,60 \times 0,185}{2 \times 0,08 \times 0,2078} = 11,8 \text{ kg/cm}^2$$

od oslonca do osnovne prečeline podignuti
na $h = 10 \text{ cm}$ po sebi

$$\bar{C}_{B_d} = 17,2 \text{ cm}$$

I dijagram, C tako da je:



2. SREDNJA TEZINA GRČE

2. SREDNJA TEZINA GRČE

$$Z_1 = \frac{12,6 + 10,8}{2} \times 262 \times 10 \times \frac{1}{52} = 23200 \text{ kg}$$

$$f_{us} = \frac{23200}{1000} = 16,60 \text{ cm}^2$$

$$\phi 12/16 \text{ cm} \quad f_{us} = 19,60 \text{ cm}^2$$

$$Z_2 = \frac{12,9 + 10,9}{2} \times 262 \times 10 \times \frac{1}{52} = 17600 \text{ kg}$$

$$f_{us} = \frac{17600}{1000} = 17,60 \text{ cm}^2$$

$$\phi 10/15 \text{ cm} \quad f_{us} = 12,00 \text{ cm}^2$$

Po cijelom dijelu u #3121 je potreban
PODVAČ

PODVAČ

"U početku pravcu našeg se predstavlja
važeći je raspored dimljena grčda ran-
čega se izračunje traka širine 1m.

Prikazom rasporeda učit će da su
navedeni:

$$\text{u dolju: } M = + \frac{l^2}{16}$$

$$\text{u dobrodošli: } M = - \frac{l^2}{12}$$

IDA JE DISPOZICIJA!



GRČE SE VEDI

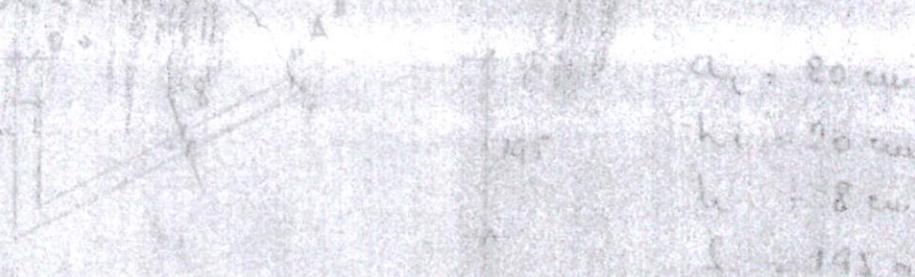
$$Z_1 = \frac{1}{6} \cdot \frac{3.53}{6.00} + 6.585 \text{ t/m}^3$$

$M_{\text{kg}} = 0,385 \times 0,344 \text{ t/m}^3 \text{ m}^3$

$$M_{\text{kg}} = 0,385 \times \frac{3,0}{12} = 0,962 \text{ t/m}^3$$

$$D = D_0 + 0,385 \times \frac{3,0}{2} = 0,83 \text{ t/m}^3$$

РЕКЛАСИЯ СИТИАНОД 410121 СТQ. 234 1335 код
КОАЦИОНАР ДОЛЖНА БЫ ПРОВЕДАТЬ НЕ МЕНЕЕ
В СОВОДОМ УДАЧНОСТЬ РОБОТЫ А ПРЕДУ
СЕГО ДОДЕЛЬНО



Использование танка для хранения 30 м³ воды

$$\frac{M}{V} = 0,30 \quad V = 10 \text{ m}^3$$

$$\frac{M}{V} = 0,45 \quad V = 6,66 \text{ m}^3$$

Использование танка

$$\frac{M}{V} = 0,41 \text{ где } k = 1,50 - \frac{M}{V} \times 0,50 = 1,14$$

Использование танка для хранения

$$V = 10 \times 0,418 = 4,18 \text{ m}^3$$

Использование танка для хранения

$$V = \frac{1,00 + 0,08^2}{6} = 0,00107 \text{ m}^3$$

Использование танка для хранения

$$V = \frac{0,341 + 0,08^2}{6,00107} = 0,00192 \text{ m}^3$$

$$G = \frac{0,490}{0,1152} = 4,27$$

$$G = \frac{0,490}{0,1152} = 4,27 \text{ t/m}^2$$

POTREBNA KONSTRUKCIJA

$$G_a \cdot 0,9h = 1,6 \times 0,9 \times 0,490 = 0,1152 \text{ N/mm}^2$$

$$- u \text{ polju } T_a = \frac{0,344}{0,1152} = 2,96 \text{ cm}^2$$

$$\rightarrow 4,27/\text{m}^2 \cdot 2,96 = 12,01 \text{ cm}^2$$

$$\text{DODJEKA } G \cdot G/\text{m}^2 \cdot S \cdot T_a = 1,68 \text{ cm}^2$$

$$- u \text{ polje } T_a = \frac{0,490}{0,1152} = 3,42 \text{ cm}^2$$

POSTOJI IZ POLJA

$$(3+5) \cdot 1,68/\text{m}^2 \cdot 3,42 = 3,69 \text{ cm}^2$$

$$G \cdot G/\text{m}^2 \cdot S \cdot T_a = 1,68 \text{ cm}^2$$

$$S \cdot T_a = 4,69 \text{ cm}^2$$

$$\text{DODJEKA } G \cdot G/\text{m}^2 \cdot S \cdot T_a = 1,68 \text{ cm}^2$$

- u gornjem

$$T_a = \frac{0,490}{0,1152} = 4,24 \text{ cm}^2$$

ZADOVOLJAVAJUĆA KONSTRUKCIJA KAO UGD SREDNJI

OSNOVNI

POČNA ČITIRAKOM KUDIĆI STRUKU 335 + 230 = 20

PROŠIĆI POKLJUCA

UVEĆAVAJUĆI TALASI POKLJUCAVAJUĆE KAO Polu
TALASI TE POKLJUCAVATE UBOGD KAO SREDNJI

TALASI POKLJUCAVATE UBOGD KAO SREDNJI

NETOGRAD

4720

PRESENT

80

100

60

40

Uzimajući u obzir da je $\eta = 0.8$

$$J_1 = Q_{\text{voda}} \cdot t = 16,0 \text{ m}^3 \cdot 12 \text{ s}$$

$$J_1 = 19,2 \text{ dm}^3$$

$$J_2 = 4,0 \text{ dm}^3$$

$$J_2 = \frac{1}{12} \times 2,0 \times 8,0 = 1,33 \text{ dm}^3$$

TALASADA *

$$T_c = 72,30 \text{ dm}^3$$

$$y_c = 7,7 \text{ dm}$$

$$y_c = 26,98 \text{ dm}^3$$

Opredelenje *

a. Za vodne moste

Jednako podjelje

$$1. \text{ Ob poprečne prečice } 0,23 + h_m$$

$$2. \text{ Moste } 0,20 \times 0,40 \times 2,50 = 4,00 \text{ m}^3$$

$$Q_1 = 17,3 \text{ l/s}$$

Koncentracije *

$$3. \text{ Difragni mostovi } \frac{1}{2} \times 1,20 = 6,66 \text{ l/s}$$

$$4. \text{ Zategna stala } \frac{1}{2} \times 17,40 = 8,70 \text{ l/s}$$

$$5. \text{ Zategna suga, } \frac{1}{2} \times 2,90 = 1,45 \text{ l/s}$$

$$Q_1 = 10,60 \text{ l/s}$$

b. Za talas

Jednako podjelje

$$1. \text{ Ob poprečne prečice } 2 \times 0,83 = 1,66 \text{ l/s}$$

$$Q_2 = 1,66 \text{ l/s}$$

UTICAJI *

Zavrsni most

Nekoliko različitih

$$B_3 = -1,23 \times \frac{20,50^2}{2} + 1,66 \times 3,60 = -3,60 \text{ t}$$

$$B_2 = -1,23 \times 5,25 + 40,80 = -17,10 \text{ t}$$

4Δ GEDDI

$$\text{SPOTZUSSILO } A_3 = -B_3 = -\frac{-17,10}{20,50} = -3,60 \text{ t}$$

$$\Delta d = (1,23 \times \frac{20,50}{2} + 3,60) = 9,0 \text{ t}$$

$$B_1 = -(1,23 \times \frac{20,50}{2} + 3,60) = -46,2 \text{ t}$$

$$A = 9,0 \text{ t}$$

$$B = 46,2 + 17,10 = 33,4 \text{ t}$$

$$\text{WEE } M_C = \frac{9,00^2}{2 \times 1,23} = 33,00 \text{ kNm}$$

$$X = \frac{2 \times 9,0}{1,23} = 14,50 \text{ m}$$

6. ZADOLDS

4Δ KUDZOLI

$$M_{BC} = -1,66 \times \frac{5,25^2}{2} + 23,00 \text{ Nm}$$

$$B_{DC} = 1,66 \times 5,25 = 8,80 \text{ t}$$

4Δ GEDDI

$$\text{SPOTZUSSILO } A_3 = -B_2 = -\frac{23,00}{20,50} = -1,10 \text{ t}$$

$$\Delta d = (1,66 \times \frac{20,50}{2} - 1,10) = -15,90 \text{ t}$$

$$B_1 = -(1,66 \times \frac{20,50}{2} + 1,10) = -18,10 \text{ t}$$

$$A = 15,90 \text{ t}$$

$$B = 18,10 + 8,80 = 26,90 \text{ t}$$

$$\text{WEE } M_C = \frac{15,90^2}{2 \times 1,66} = 76,0 \text{ kNm}$$

$$X = \frac{2 \times 15,90}{1,66} = 19,10 \text{ m}$$

$$\left(\frac{1}{\beta} + \frac{\alpha}{\gamma_c} + \frac{\alpha^2}{\gamma_c^2} \right) T + \frac{G_M}{\beta \gamma_c} = \frac{16.0}{0.16} = 100$$

$$\left(\frac{1}{0.16} + \frac{0.16}{0.16} + \frac{0.16^2}{0.16^2} \right) T + \frac{6 \times 33.0}{0.16 \times 0.16} = \frac{76.0}{0.16} = 480$$

$$(25.0 + 2.8 + 5.7)T + 1380.0 = 280.0 + 0$$

$$T = \frac{480.0}{5.7} = 84.5$$

Uganda S.U.

Uganda S.U.

$$G_0 = \frac{4c(\beta_i - \gamma_c)}{\gamma_c} = \left[\frac{2}{\gamma_c} - 2\gamma_c(\beta_i - \gamma_c) \right] T$$

$$\beta_i - \gamma_c = 1.95 - 0.77 = 1.18$$

$$G_0 = \frac{16.0 \times 1.18}{0.16} = \left[\frac{2}{0.16} - \frac{2 \times 0.77 \times 1.18}{0.16} \right] \times 32.8$$

$$X_0 = -432 - (2.8 + 5.7) \times 32.8$$

$$G_0 = -432 + 192 = -240 \text{ Hn}^2$$

Uganda S.U.

$$G_u = \frac{4c(\beta_i)}{\beta_i} + \left(\frac{1}{\beta_i} + \frac{\alpha^2}{\alpha^2} \right) T$$

$$G_u = \frac{33.0 \times 0.40}{0.16 \times 0.16} + \left(\frac{6}{0.16} + \frac{0.16^2}{0.16 \times 0.16} \right) \times 32.8$$

$$G_u = 15.0 + 8.0 = 23.0 \text{ Hn}^2$$

Pracovní síla je $F = 16,2 \text{ kN}$. Výška závěsu je $h = 1,5 \text{ m}$.
Výška závěsu je $h = 1,5 \text{ m}$. Pracovní síla je $F = 16,2 \text{ kN}$.
Pracovní síla je $F = 16,2 \text{ kN}$. Výška závěsu je $h = 1,5 \text{ m}$.

$$T = F \cdot h = 16,2 \times 1,5 = 24,3 \text{ kN}$$

$$G = 16,2 \text{ kN}$$

$$g = 9,81 \text{ m/s}^2$$

$$\frac{T}{G} = \frac{24,3}{16,2} = 1,515 \approx 1,5$$

Smečka sítia je:

$$(25,0 + 2,8 + 1,7) \text{ m} = 30,5 \text{ m}$$

$$T = \frac{150}{3,1} = 48,4 \text{ kN}$$

Na počítaču:

$$G_0 = -432 - (2,8 - 3,7) \times 48,4 \text{ kN}$$

$$G_0 = -432 + 96 = -336 \text{ kN/m}^2$$

Na počítaču výpočet výkonu:

$$G_0 = \frac{33 \times 0,55}{0,0224} \cdot \left(\frac{1}{0,22} + \frac{0,55^2}{0,0224} \right) \times 48,4$$

$$G_0 = 870 + 300 = 1170 \text{ kN/m}^2$$

Na počítaču výpočet výkonu:

$$G_0 = -870 \cdot 16,2 = -14244 \text{ kN/m}^2$$

$$G_0 = -870 \cdot 16,2 = -14244 \text{ kN/m}^2$$

Dnešní dílčí výpočet výkonu výpočet výkonu:

$$\chi_1 = \frac{103.0}{129.0} = 0.85 \sim 1.0$$

Zarecaurus ornatus (Gmelin) in India.

$$Bl = 16, \bar{f} = 18, n = 2, d = 3, t$$

ARM SEED Polygalaciliata

$$C = \frac{24.3}{44.20} + 0.78 \approx 0.50$$

DATA: 50% O HABITACO A 70% U VIEZ MUS

600-14511 - ACROSS N. 10249

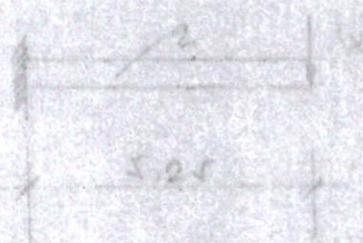
OPTICAL ENGINEER

A. Hessö

$$g = 1.23 \text{ cm}^{-1}$$

3. On 25th Dec.

(3) 40.80 kJ/m



$$M_A = -\left(1,23 + \frac{5,25^2}{2} + 10,80 \times 5,25 \right) = -72,8 \text{ kN}$$

$$\lambda = 4.23 \times 5.25 + 10.40 = 47.306$$

100
✓ 7010000
100

Aud.

400 - 0.295
348

čvor 165 / 4-30

$$Z_A = 1724 \text{ cm}^3 \quad A = 26,10 \text{ cm}^2 \\ 1,4 \times 0,9 \times 1,0$$

čvor 165 je u potpunosti u obliku $F_A = G_{A,0}$ cm²
širina je 16,10 cm

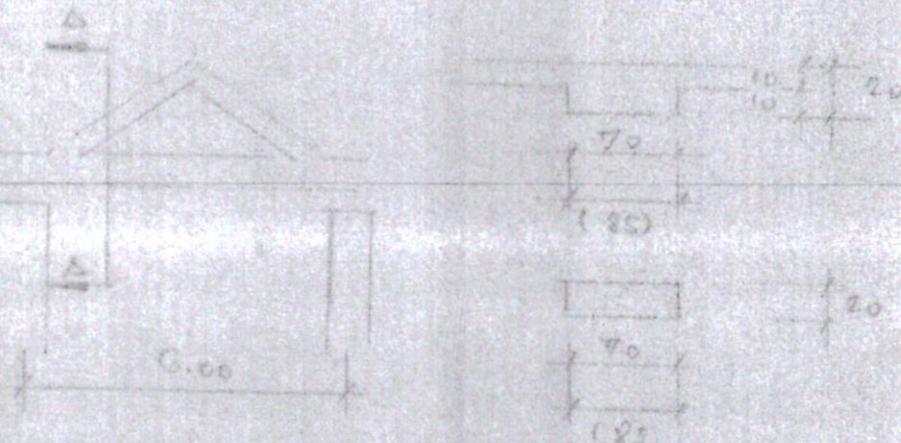
$$C_A = 17304 \text{ kg/m}^2 \quad g = 9,81 \text{ m/s}^2 \quad 7700 \text{ kg/m}^3$$

Počinjajući u vidu što je u jednom
diagonali su opterećene oblasti
pristupne uobičajenoj raspodjeli sile.
Uvođenje strukture slike:

$$F_A = 46,0 \text{ kg/cm}^2$$

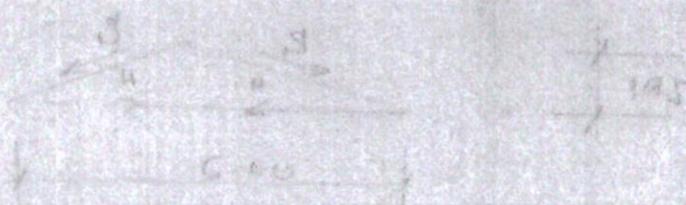
$$S_x = \frac{16,0 \times 3,61 - 12}{2} = 30 \text{ cm}^2$$

Dimenzije dijagonale po slici:



Dimenzije 25 su da pogoduju fiksabu a
70 za zatvornu i oštriju

čaptički sistgi u 26.



$$A_0 = 711.63 \text{ ft}^2$$

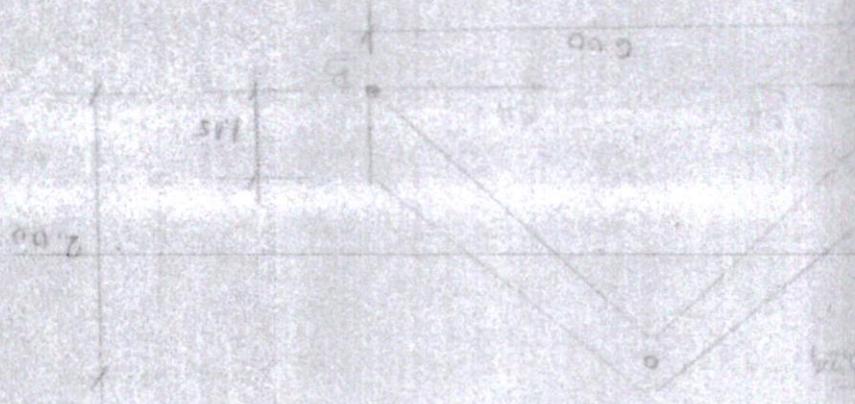
$$S_1 = 5,225 \text{ ft}^2 \quad (0.05 \text{ Pcs. } 4/21)$$

$$L_0 = 0.019 \times 6.0 \text{ ft} = 0.114 \text{ ft}$$

$$L_0 = 8.6 \cdot 0.114 \times 3.06 = 0.65 \text{ ft}$$

UTIICAL OD "A"

$$Q_0 = 0.30 + 0.15 = 10.12 \times 2.80 = 0.219 \text{ ft}^3/\text{sec}$$



$$E_a = \frac{A_0 \times 0.9 \times 0.5}{2} = \frac{30.0 \text{ cu ft}}{2} = 15.0 \text{ cu ft}$$

$$n = 0.05 \times \frac{8}{6.00} = 0.067$$

$$q = 0.20 \times 0.7 \times 2.80 = 0.392 \text{ ft}^3/\text{sec}$$

$$H = 5 \text{ CSDA} = 35.6 \times 0.84 = 29.44$$

$$Q = 0.30 \times 0.5 \times 2.80 \times 0.84 = 0.36 \text{ cu ft/sec}$$

Q = 0.36 cu ft/sec

$$H = 0,6 \cdot 0,05 + 0,67 = 0,33 \text{ m}$$

$$H = 0,6 \cdot 0,05 + 0,67 = 0,33 \text{ m}$$

$$H = 0,6 \cdot 0,05 + 0,67 = 0,33 \text{ m}$$

$$\epsilon_1 = \frac{3,21}{3,56} = 0,90 \text{ m}$$

$$d'_{16} = 92 \text{ cm}$$

$$H_a = 3,56 \left(0,05 + \frac{0,67 - 0,33}{2} \right) = 4,32 \text{ m}$$

$$n = \sqrt{\frac{69}{452 \cdot 0,69}} = \sqrt{\frac{69}{196}} = 0,352$$

$$G_{67} < 105 / 1400 \text{ kg/cm}^2$$

DOTOCZNA ARMATURA

$$T_a = \frac{4,32}{1,65 + 0,69} = \frac{3,54}{1,6} = 2,20 \text{ cm}$$

Konstrukcja z 1 m. dzierżawy na 3 cm kompozyt
konstrukcyjny, węzły sklejki - umieszczone

Poziomy nosceń konstrukcji - 0,02000 m²

$$T_a = 1,00 \text{ cm}^2$$

Mocowanie sklejki ukośne - 0,0400 m²

$$T_a = 1,00 \text{ cm}^2$$

Tło pod sklejką mocowane umieszczone 40 cm.

OD 4 8/20 SLUŻY WHEŚCIE W GŁÓWCE

• 3. Punkt der 6 VDE-Mittelwerte

• 3. Vom 2. Block aus dem Mittelwert
• 1. Standardabweichung
• 1. 6,03 mm

PICOCCESE 1

120 LADEN

a. ESTUARIA

b. BASIC BATH

c. STEWARTON

d. MIRSAN BOTON

e. ANDAMAN

f. RABIA PLATOU

g. SINGAPORE

Bo = 1

2.50

7.0

1.00

q = 660 kg/m³

$$q_{12} = 660 \times 0.4 = 264 \text{ kg/m}^3$$

$$H = 2 \frac{c^2}{M} = 264 \times \frac{6.0^2}{11} = 852 \text{ kg/m}$$

$$T_A = \frac{1}{14.00 \times 0.9 \times 1.1 \times 2} = 0.52 \text{ cm}^2$$

$$A = 1.412 \times 1.0 \times 1.1 \text{ cm } T_A = 2.68 \text{ cm}^2$$

TIP_n(1)

$$A = 2 \cdot \frac{c}{2} \times 660 \times \frac{6.0}{2} = 6930.0 \text{ kg}$$

• 3. Punkt der 6 VDE-Mittelwerte

• 3. Vom 2. Block aus dem Mittelwert
• 1. Standardabweichung
• 1. 6,03 mm

• 3. Punkt der 6 VDE-Mittelwerte

• 3. Vom 2. Block aus dem Mittelwert
• 1. Standardabweichung
• 1. 6,03 mm

WYSOKOŚĆ PŁATNIKI W DŁUŻYNA

1,5 m, 1,6 m, 1,7 m, 1,8 m, 1,9 m, 2,0 m

1,5 m, 1,6 m, 1,7 m, 1,8 m, 1,9 m, 2,0 m

OPERACJE DLA:

$$1. \text{ SP KUWA} \quad 430 \times 0,80 = 344 \text{ kg/m}$$

$$2. \text{ SPOLSKA} \quad 6,60 \times 0,30 \times 2150 = 369 - 0,7$$

$$344 \text{ kg/m}$$

OPERACJE DLA SPOLSKA

$$\text{W POLU} \quad H = 641 \times \frac{6,60^2}{16} = 1440 \text{ kg/m}$$

$$F_a = \frac{1440}{1440 \times 0,9 \times 0,27} = 4,12 \text{ cm}^2$$

$$5 \times 12 = F_a = 5,65 \text{ cm}^2$$

(Pov. 2/12)

$$= \text{DO OSZCZĘDZANIA} \quad M_1 = C_{111} \times \frac{4,12}{16} = 2320 \text{ kg/m}$$

$$F_a = \frac{2320}{1440 \times 0,9 \times 0,27} = 6,85 \text{ cm}^2$$

$$\text{POZIOM 1 PÓŁ 1/2} \quad 6,85 \text{ cm}^2 \leq F_a \leq 142$$

$$\text{WŁAŚCIWOŚĆ} \quad 2,7/12 \leq F_a \leq 5,25$$

$$F_a = 6,77 \text{ cm}^2$$

$$\text{WŁAŚCIWOŚĆ} \quad 6,77/(30+15) = 0,17$$

WZÓRCE SU MO JEST WŁAŚCIWOŚĆ STWARTY 0,17

POBI Ewaluacyjna PRZEDMIA WŁAŚCIWOŚĆ 0,17

$$P_1 = C_{111} \times \frac{6,77}{2} = 1932 \text{ kg/m}$$

$$Q_1 = 2 \times 1932 = 3864 \text{ kg}$$

Post 1/300 Serklaż (ilużna gospa)

WŁAŚCIWOŚĆ PROW Z PÓŁKI

$$R = 6,00 \text{ m}$$

1. Osnovna površina $0,5 \times 0,80$

2. Deltoida $0,70 \times 0,90 \cdot 3,20$

3. Trapez

894 kg/m^2

Ulica 12

U Poljtu M: 894 $\frac{600}{700}$ = 2 m²/m

$$T_a = \frac{2,00}{14 \times 0,4} = 0,69 \text{ m}^2$$

$$7,00 \cdot 5,00 \cdot T_a = 7,00 \text{ m}^2$$

Pov. Ad 12

M: 47. 2412

Na osnovu M: $894 \times \frac{5,00}{14} = 2,68 \text{ m}^2$

$$T_a = \frac{2,68}{14 \times 0,4} = 0,48 \text{ m}^2$$

Dvorac: $Dov. (4+4) \times 12 \cdot 5,00 \cdot T_a = 10,08 \text{ m}^2$

M: 47. 2412 $5,00 \cdot T_a = 2,70 \text{ m}^2$

$$T_a = 12,30 \text{ m}^2$$

vezanje 48/15cm

Pos H/4

Dovolj sekvlaž

KONTAKTET PREKO 8 POLJA

l = 6,00 m

Gospodarstvo

1) Zid ispunje od supje opeke

$$0,38 \times 1,50 \times 1400 = 300 \text{ kg/m}^2$$

2) Sevulaži

$$= 300 \text{ kg/m}^2$$

$$g = 1100 \text{ kg/m}^3$$

$$\eta = \frac{q_4}{q_3} = \frac{1400}{644} = 1,72$$

Pracovní výška 2100 mm

$$H = 172 \times 2300 = 2480 \text{ kg/m}$$

$$T_a = 172 \times 2300 = 7,80 \text{ m}^2$$

$$\lambda_f A = 8,7 \text{ m}^2 \cdot \text{K} \cdot \text{W}^{-1}$$

Povídáno 2010

Na oslonu

$$H = 172 \times 2300 = 39,00 \text{ kg/m}$$

$$a = \sqrt{\frac{0,7}{2000}} = 0,035 \text{ m}$$

$$T_a = 172 \times 0,035 < 7,80 \text{ m}^2 \cdot \text{K} \cdot \text{W}^{-1}$$

$$T_a = 172 \times 0,85 = 14,70 \text{ m}^2$$

Počítání Povídáno 12x2,14x14,56 = 4,16 m = 6,16 m

$$M_{\text{osl.}} = 2,4 \cdot 0,85 \text{ S} \cdot T_a = 2,26 \text{ m}$$

$$\text{DOD} = \lambda_f \cdot 16 \cdot S \cdot T_a = 3,08 \text{ m}$$

$$S + a = 14,50 \text{ m}$$

Pohledy

$$Q_1 = 172 \times 1932 = 3340 \text{ kg}$$

$$Q_2 = 2 \times 5310 = 6680 \text{ kg}$$

Smitecí nadoba

$$C = \frac{3340}{60 \times 0,9 \times 4} = 5,56 \text{ kg/m}^2 < 7,80 \text{ kg/m}^2$$

b2c4c7je

$$40 \times (30 + 15) =$$

Uzavřené surové sklo 4500 kg / 5000 sklo svařovací

OLNICKA RADÍČEK - ALHOT - VYJEDNAVACÍ MĚSÍČ

10.10.2019 00:00

1. IZVRŠENJE

U DOLINIČNU UMETJENJU

(K-PROVJERJENO OD 4,70 - 10,50 m)

(I-PROVJERJENO OD 5,10 - 10,90 m)

Raspotri podzemnih vodovoda

5-1	$l =$	5,16 m
5-2	$l =$	5,82 m
5-3	$l_c =$	6,35 m
5-4	$l_c =$	7,28 m
5-5	$l_c =$	8,00 m
5-6	$l_c =$	8,72 m
5-7	$l_c =$	9,45 m
5-8	$l_c =$	10,18 m
5-9	$l_c =$	10,90 m

OPTEREDCENJE (Pos H/5-2 do Pos H/5-8)

$$1. \text{ od Pos H/2} \quad 2 \times 1980 = 3960 \text{ kg/m}^3$$

$$2. \text{ PODVLAKA } 0,40 \times 0,60 \times 2.400 = 600 \text{ "}$$

$$q = 4.560 \text{ kg/m}^3$$

UKLJUČENJE DELIMIČNU VZETU JE

UKOZSMANDJENJE MJEOMA U DOLJU 4,5

$$M = q \frac{l_c}{10}$$

ODTEREDCENJE (Pos H/5-1 i Pos H/5-9)

$$1. \text{ od Pos H/2} \quad = 1980 \text{ kg/m}^3$$

$$2. \text{ PODVLAKA} \quad = 600 \text{ "}$$

$$= 2.580 \text{ kg/m}^3$$

PRODAĆU IZVRŠIH 4A SLEDEĆI RADNI *

1) RACUNATA JE POS H/5-8 SA NEVĆIM RASPOTROM PO
TE ARMATURA HADJERA KOB FIG. ZA OSTALE A 150
MM: DGA VCIJE.

a. Pos 4/5-9

KOEFFICIJEHTI

$$\eta_1 = \frac{q_{11} \times \left(\frac{l_9}{l_8}\right)^2}{q_1} = \frac{2,58}{4,56} \times \left(\frac{10,90}{10,48}\right) = 0,658$$

$$\eta_2 = \frac{q_{11} \times \frac{l_9}{l_8}}{q_1} = \frac{2,58}{4,56} \times \frac{10,90}{10,48} = 0,607$$

ARMATURA

$$f_w = 0,658 \times 07,5 = 44,2 \text{ cm}^2$$

DOL. 1/3 + 2/5 RASPODA

REAKCIJE

$$R_1 = 0,607 \times 23,20 = 14,10 \text{ t}$$

UZETE UZOREK 48(30+10) cm. ODOBRAJENI

NA P/5 OD OSLOHCA

b. Pos 4/5-8

$$M = 4,56 \times \frac{10,18^2}{80} = 47,4 \text{ t}$$

$$R_1 = 4,56 \times \frac{10,18}{2} = 23,2 \text{ t}$$

OBDODENI DA UNIJEŠTEĆE MOSETA, AWDAN

MOSET SE UZETI, b = 100 cm. KOD PODRACUNA N

KETONAT 56, PA JE

$$n = \frac{55}{\sqrt{47400}} = \frac{55}{218} = 0,252$$

$$6 \text{ b/t} < 105 / 1400 \text{ kg/m}^3$$

ARMATURA

$$f_w = \frac{47,4}{1,4 \times 0,25 \times 0,55} = 67,50 \text{ cm}^2$$

SLIČNUĆI UZ POŠTI

$$C = \frac{23200}{40 \times 0,9 \times 55} = 12,0 \text{ kg/cm}^2$$

$$f_{ax} = 12,0 \text{ N/mm}^2 \quad f_{ck} = 40 \text{ N/mm}^2$$

$$x_c = 12,0 - 7,5 = 4,5, 0,9 = 2,32 \text{ mm}$$

$$S = \frac{12,0 + 7,5}{2} \times 2,32 \times \frac{40}{12} = 64000 \text{ kg}$$

$$I_{KS} = \frac{Gh_000}{1400} = 46 \text{ cm}^4$$

ZA POKRIVANJE KROSE SILE ZA DUSIČI

$\lambda = 2,32 \text{ mm}$ OD OSLOUCA

$$\text{Pov. } \cdot \frac{1}{3} I_a = \frac{1}{3} \times 67,5 = 22,5 \text{ cm}^2$$

$$\text{UZECIČ. } \phi 8/10 \text{ mm} \quad I_a = \frac{23 \times 2 \times 1,0}{1,04} = 32,0 \text{ cm}^2$$

$$I_{KS} = 54,5 \text{ cm}^4$$

"A OSÍALON DELU UZECIČE"

$\phi 8/80 \text{ mm}$

C. Pos H/5-7

- NOTIFICATION -

$$\eta_1 = \left(\frac{l_9}{l_8} \right)^2 = (0,93)^2 = 0,87$$

$$\eta_2 = \frac{l_9}{l_8} = \frac{9,65}{10,18} = 0,93$$

ADMATURA

$$f_a = 0,87 \times 67,5 = 59,0 \text{ mm}^2$$

Pov. $1/3$ na l/5 od oslonca

Brez ugaže $\eta = 1/(30 \cdot 10) \text{ cm} = 0,0333 \text{ m}$
10 cm na l/s od stolica

de Post 4/5-6

vložičenje

$$\eta_1 = \left(\frac{l_6}{l_8} \right)^2 = (0,86)^2 = 0,75$$

$$\eta_2 = \left(\frac{l_6}{l_8} \right) = \frac{8,72}{10,18} = 0,86$$

ADMATURA

$$F_a = 0,75 \times 67,5 = 50,6 \text{ cm}^2$$

Pov. $1/3$ na l/s od stolica

REAKCIJA

$$R_1 = 0,86 \times 23,2 = 20,0 \text{ t}$$

brez ugaže $\eta = 1/(30 \cdot 10) \text{ cm} = 0,0333 \text{ m}$

10 cm na l/s od stolica

e Pos 4/5-5

vložičenje

$$\eta_1 = \left(\frac{l_5}{l_8} \right)^2 = (0,785)^2 = 0,615$$

$$\eta_2 = \frac{l_5}{l_8} = \frac{8,72}{10,18} = 0,785$$

ADMATURA

$$F_a = 0,615 \times 67,5 = 41,6 \text{ cm}^2$$

Pov. $1/3$ na l/s od stolica

REAKCIJA

$$R_1 = 0,785 \times 23,2 = 18,3 \text{ t}$$

1. krok - výpočet průtoku v horní části kanálu

f. 3,000 A/5,1

KONSTRUKCE:

$$M_1 = \left(\frac{l_4}{l_8}\right)^2 = (0,715)^2 = 0,512$$

$$M_2 = \frac{l_4}{l_8} = \frac{7,28}{10,18} = 0,715$$

KONSTRUKCE:

$$T_a = 0,512 \times 67,5 = 34,8 \text{ cm}^2$$

Dov. M_3 na $\ell/5$ od stolu

PLAKETKA:

$$D_1 = 0,715 \times 23,2 = 16,6 \text{ t}$$

UZEMNĚTE $\phi 8/(30 \text{ t})_{\text{m}}$, GASTRONEZ

10 cm na $\ell/5$ od stolu.

G. Pos H/5+3

KONSTRUKCE:

$$M_1 = \left(\frac{l_3}{l_8}\right)^2 = (0,642)^2 = 0,418$$

$$M_2 = \frac{l_3}{l_8} = \frac{6,75}{10,18} = 0,642$$

KONSTRUKCE:

$$T_a = 0,418 \times 67,5 = 28,2 \text{ cm}^2$$

Dov. M_3 na $\ell/5$ od stolu

PLAKETKA:

$$D_1 = 0,642 \times 23,2 = 15,0 \text{ t}$$

UZEMNĚTE $\phi 8/(30 \text{ t})_{\text{m}}$, GASTRONEZ 10 cm
na $\ell/5$ od stolu.

1. Постройте

- схему опорного контура

$$M_1 = \left(\frac{r_2}{l_2} \right)^2 \cdot (0,57)^2 = 0,325$$

$$M_2 = \frac{l_2}{r_2} = \frac{5,22}{10,18} = 0,57$$

АДМАТУРА

$$F_a = 0,325 \times 67,5 = 22,0 \text{ см}^2$$

Ров. 1/3 на 1/5 до ослонца

Расчеты

$$D_1 = 0,57 \times 23,2 = 13,2 \text{ т}$$

Брусья д/8 (20+40) см обшивка 10 см

на 1/5 до ослонца

1. Постройте

коэффициент

$$M_1 = \left(\frac{l_1}{r_2} \right)^2 = (0,467)^2 = 0,218$$

$$M_2 = \frac{l_1}{r_2} = \frac{5,10}{10,90} = 0,467$$

АДМАТУРА

$$F_a = 0,218 \times 44,2 = 9,70 \text{ см}^2$$

Ров. 1/3 на 1/5 до ослонца

Расчеты

$$D_1 = 0,467 \times 14,10 = 6,60 \text{ т}$$

Брусья д/8 (20+40) см обшивка 10 см
на 1/5 до ослонца.

Довити 1/6.1 - КРАТКА РОВЛЮКА

$l_0 = 2,5 \text{ м}$

$l_1 = 2,25 \text{ м}$

a. Роз 4/6.1 - КРАТКА РОВЛЮКА

$$q = 2,58 \text{ т/м}^2 \quad (\text{Роз 4/6.1})$$

b. Роз 4/6.2 - КРАТКА РОВЛЮКА

$$q = 4,86 \text{ т/м}^2 \quad (\text{Роз 4/6.2})$$

Розрахунок залізобетонних

(б/к = 40/30 см)

$$H = 4,56 \times \frac{3,00}{4,0} = 3,62 \text{ м}$$

$$T_a = \frac{5,60}{40 \times 0,3 \times 0,4} = 16,00 \text{ см}^2$$

$$D_1 = 4,56 \times \frac{3,00}{2} = 6,84 \text{ т}$$

$$\tau = \frac{3,00}{40 \times 0,3 \times 0,4} = 0,33 \text{ кг/см}^2$$

Довити 1/6.1 - КРАТКА РОВЛЮКА

УЗАГАДАННЯ 1/6.15см - КРАТКА РОВЛЮКА

- КРАТКА РОВЛЮКА

АРИАТИКА

$$T_a = \frac{2,16}{4,56} \times 16,00 = 9,16 \text{ см}^2$$

Редукція

$$D_1 = \frac{2,16}{4,56} \times 8,00 = 4,17 \text{ т}$$

АРИАТИКА ДОВІДАТИ У СБ МОНІТОРІНГІДЕ РІВНЯННЯ

на 640136 4/6/20 см

Slope = 10% - 1000 m	
1. Trop. forest	15 kg/m ³
2. Evergreen sub	0.30 - - -
3. Transition	0.15 - - -
4. Cerrado	0.10 - - -
	q = 780 kg/m ³

$$f = 780 \times 0.4 = 312 \text{ kg/m}^3$$

$$M = 780 \times \frac{6.0}{2} = 2340 \text{ kg}$$

$$F_a = \frac{A_1 f}{1400 \times 0.023} = 2.08 \text{ cm}^2$$

$$2.08 \times 2.08 = 4.30 \text{ cm}^2$$

- 7.10 " U'

Q = A U C D P = ?

$$A = 780 \times 6.0 = 2340 \text{ kg}$$

C = 0.00000002 - 0.00000002

P = 4/8 - (0.00000002) 4.30

$$U = 1.20 \quad U = 1.05 + 1.70 = 1.260$$

Q = 0.00000002

$$q = 780 \text{ kg/m}^3 \quad (\text{Pos } 4/7)$$

$$M = 780 \times \frac{6.0}{2} = 618 \text{ kg}$$

$$f = 780 \times 1.70 = 990 \text{ kg/m}^3$$

$$n = \frac{2.08}{\sqrt{618}} = \frac{2.08}{2.48} = 0.840$$

Stress = 0.840 kg/cm²

$$F_a = \frac{6.0}{1400 \times 0.00000002} = 5.99 \text{ cm}^2$$

1. 10. 1978
SLOVENSKO / 3/15 m.

Vlo. 4/9 - SERVILÁZ ŽIVČEHO. HOLE
KONTAKTURIT PREDNÍ & POLE
L = 6.00m

OPTEREDENDE:

$$\begin{aligned} 1. \text{OD TSUAVICE} & 330 \times 0,20 = 264 \text{ kg/m} \\ 2. \text{SERVILÁZ} & 0,0080,30 \times 2500 = 300 \text{ kg/m} \\ 3. \text{OD PoS 4/8} & = 494 \text{ kg/m} \end{aligned}$$

$$Q = 1554 \text{ kg/m}$$

UTICAJI I DIMENZIJSKAJNE

- u Dolzu i slobodnočinu

$$M = 1554 \times \frac{6,00}{10} = 5610 \text{ kgf}$$

$$R_1 = 1554 \times \frac{6,00}{2} = 4662 \text{ kgf}$$

$$R_2 = 2 \times 4662 = 9324 \text{ kgf}$$

$$q = \frac{27}{\sqrt{163500}} = \frac{27}{113} = 0,227$$

$$q = 0,227 \text{ kgf} = 105 / 4500 \text{ kgf/m}^2$$

$$C = \frac{4662}{40 \times 0,9 \times 0,7} = 4,8 \text{ kg/m}^2$$

ADMATUDA:

$$J_{st} = \frac{5610}{(40 \times 0,9 \times 0,7)} = 16,6 \text{ cm}^3$$

u polju pov. 1/3

u do osloučnib dodati 1/2 užitkového
 $\phi_c / (30 + 15) \text{ cm} = 60 \times 0,0400 / 45 \text{ cm} = 1,33 \text{ cm}$
u do osloučnib

1.00 - Zeleni beton
Koeficijent bez pogreške
 $l = 600 \text{ mm}$

OPTEREDENJE

1. 00 TAVADICE	= 264 kg/m ³
2. SERVLAŽI	= 300 kg/m ³
3. 00 ZID 0,12 x 4,5 x 1700	= 930 kg/m ³
	$q = 1494 \text{ kg/m}^3$

$$\text{KAKO } 36 \times 29 \approx 930$$

To su i uticaji i dinamiz je isti

Ovu poziciju uveo za pos 4/q

Pos 4/10a - Servlažna raspoređujuća
zidu

Dva je servlaža uvezi za alveo -
vadne stupova i leži na podložnom
zidu.

Dimenzije konstrukтивne

$$d/b = 30/40 \text{ cm}$$

Arenatura gorenje 3f 14

dole 3f 14

uzorki 4c/20 cm

Pos 4/m - Podvlaka

- Delnično uklječivanje

$l_0 = 800 \text{ mm}$ u uključujući 4,90 - 10,50 m

Ugao od 45° - od 5,40 - 10,91 m

Gospodarsko (svakodnevno)

1. 00 Pos 4/q $2 \times 2240 = 4480 \text{ kg}/\text{m}$

2. Podvlaka $\times 600$

$$q = 5080 \text{ kg}/\text{m}$$

6720.41

9. 10804-5220

1

Δ₁₋₂ = 56.057416 80.140446 0.1 - 0.026 28.0391

Dos #15 una PGCB Power Control Board para

$$d = \frac{S_{AS}}{S_{AT}} \times f_{AS} + 0.8 \text{ (approximate)} \quad \text{in mm}$$

DATA FROM PREVIOUS

Blas = Hoffm. (1862)

11 AUGUST 1942 - 7400 ft. - 600 P.M.

1971년 7월 25일 ~ 1972년 10월 26일

WADG-1 Pass #1 At -24 Deg +10 Deg 06 Min 55 Sec

Reaction:

Page 3 of 3

LEADER: $m_1 = \frac{115}{9.8} = 2.00$

$$M_2 = \frac{1.15}{6.57} = 0.18$$

$$Q_0 = 2.00 \times 14.0 = 28.0$$

$\Delta A_0 = 0.02$ at 25°C.

Q. 1 = AAD x 2500 = 25,500

86-5-A-102-2690 - 7-27-81

$R_C = 1.0 \times 10^{-3} \text{ m}^2/\text{V}$

$$P_{\text{d}} = A_s A_b \times 10^6 \text{ G} \cdot \text{m} = A_s A_b \cdot 10^6$$

Physiol. Acclim. Metab. 1964, 16, 103

10. *Leucosia* *leucostoma* *leucostoma* *leucostoma* *leucostoma*

10.000 6.600 2.700

Позиция 4/12 - Погружение в воду в течение 10 минут

Время погружения 10.00 ч.

Поз 4/12 - Погружение

Длительность погружения

10 ± 3.10 м

10 ± 3.10 м

Сумма 9 = 5.15 т/м (Поз 4/12)

Послед. Поз 4/6 - вытеснение из

Моря 100 ± 3.45 т/м

Абсолютная

Погружение в воду в течение

Длительность погружения

Длительность погружения Поз 4/6 = 5.15

Длительность

Установка в воду Погружение в воду 4.50

Установка в воду

Поз 4/13 - Ступ ладони против колодки

10 = 4.50 м

Среднее значение

a. За минуту израсход Поз 4/5.1

1. 00 Поз 4/5 = 4.83 т

2. 00 Поз 4/5 = 3.21 т

3. 00 Поз 4/5 = 3.60 т

4. 50 т

сумма 4.00 + 3.21 + 3.60 = 10.81 т

P = 13.67 т

b. За минуту израсход Поз 4/5.2

1. 1000	1. 1. 2	= 2.86 t
2. 1000	Po 1. 1. 4	= 5.64 t
3. 1000	1. 2. 5	= 23.20 t
4. 1000		= 1.20 t
		D = 35.64 t

12 HEDDUN BERPADAU INTERDUKSAWU

$$24 \mu = 2\% \text{, Fa} = 16 \text{ cm}^2$$

$$\text{ODALUNG} = 4/22 \text{ sa } \text{Fa} = 16.2 \text{ cm}^2$$

$$\text{Tbl} = 40 \times 40 \times 10 \times 16.2 = 1728 \text{ cm}^3$$

$$G = \frac{1728}{1752} = 0.974 \text{ t/cm}^3$$

Zensus di STUP MUSI BOJONEGORO
DENGAN KAPASITAS 45000 HEKTAR DENGAN

PoS H/mu = Sumber STUP

(sangat di sediakan dalam)

Jarak antar kx = 2 row

ROWANTARAKA = 400 m

Berdasarkan

a. Zona kritisasi nilai PoS H/mu

1. RAB CERAH = 0.974 PoS H/mu = 13.67 t

2. RAB RENDAH = 0.974 PoS H/mu = 11.50 t

P = 11.50 t

3.000 t = 1/10.4

45.26 - 4-

4.00 PoS H/mu

= 8.00 t

5. SUMB

= 6.20 t

P = 41.94 t

1. OD SPRATU VERO POS H/1	= 35,14
2. OD POS H/1	= 5,00
	P ₂ = 43,14
3. OD POS H/M-2	= 25,60
4. OD POS H/12	= 8,40
5. STUD	= 1,80
	P ₅ = 79,74

12 M Č DGU LÍDC AHOJU LÍČENÍ POKLADU
 ZD POCETU, KOMUNIKU VERO POS H/12
 UKOŠE

$$G = \frac{79740}{1952} = 40 \text{ týdny}$$

ZD ČI DA STUD MOZE PRIMÍT 1V4740
 JAK SU MÁ POCZTAHODA HCISKOZASÉCNA

Pos 4/15 - STUD POČNA HOLY

I SPRAT = kč = 4,50 K
 POČNA HOLY = kč = 4,50 K

a. Pos 4/15-1 V RASSADY, S TÝD.

G POCOZ CENZO

1. OD POS H/1	= 24,90 K
2. OD POS H/3	= 4,94 K
3. OD POS H/4	= 3,34 K
4. OD POS H/6-1	= 4,50 K

5. STUD

OD 0,070 + 0,10 + 2,50 = 4,67 K

P₅ = 46,67

1. 00 Pos H/1	1.00	1.00
2. 00 Pos H/2	1.00	1.00
3. STUB 6.10 x 0.70 x 1.20 x 2.00	1.00	1.00
		P _p = 55.80 t

4. Pos H/3.2 - 1000001 STUB

ODREĐENJE:

1. 00 Pos H/1	2.00	0.00
2. 00 Pos H/3	3.00	0.00
3. 00 Pos H/4	6.00	0.00
4. 00 Pos H/6-2	4.00	0.00
5. STUB		P _p = 56.78 t

6. 00 Pos H/7	9.00	0.00
7. 00 Pos H/12	8.00	0.00
8. STUB	15.00	0.00

$$P_p = 76.04 t$$

$$Z_{\Delta} \mu = 1\%$$

$$T_{\Delta} = 8422$$

$$F_{\Delta} = 90 \times 70 \times 105 \times 2.2 = 30.82 \text{ m}^2$$

DATA: 1. 5. 1974.

$$- Z_{\Delta} \text{ KRAJNJI } G = \frac{55300}{3082} = 18.42 \text{ kg/cm}^2$$

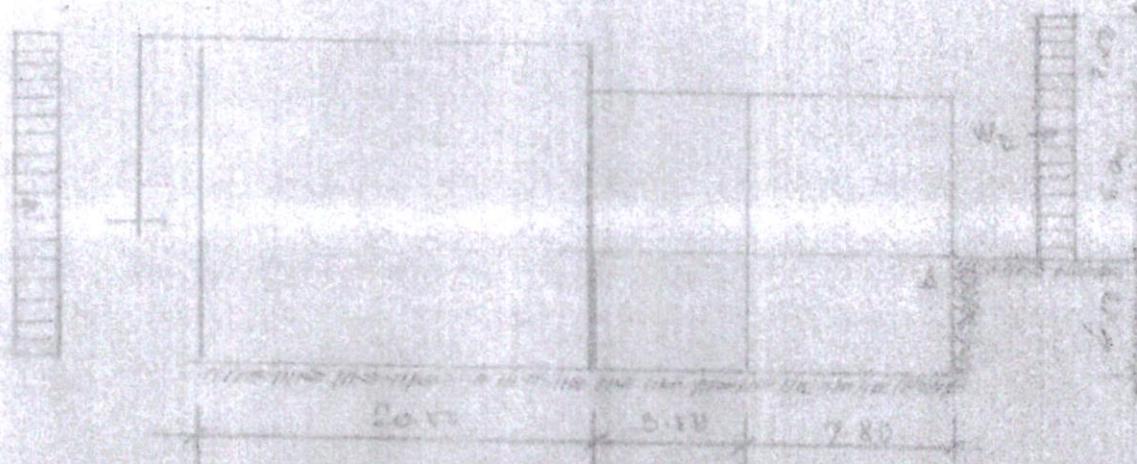
$$- Z_{\Delta} \text{ SREDNJI } G = \frac{76040}{3082} = 25.03 \text{ kg/cm}^2$$

Što znači da ima os zadržku 25.03 t/m².

POINDELJA A + 1) Svi stubovi su načinjeni
kontrolisani na včeraj iste 30.04.1974. godine.
SILVANO KAROLJKO - STROJEVNI INGENJER
D. SVE ZADNE U KRAJNJI ZAVRŠNI BROJ:

1. 10. 1970
Výkon výroby 1570 t/6 hodiny v Lounské
výrobce výrobek 800, 24. 9. 1970, 11. 10. 1970, 12.
10. 1970.

PROJEKCE VÝRODZA STUŽOVÉ AMORLA DISPOZICÍ A SISTEMU



OPTIMALIZACE VÝRODY - I

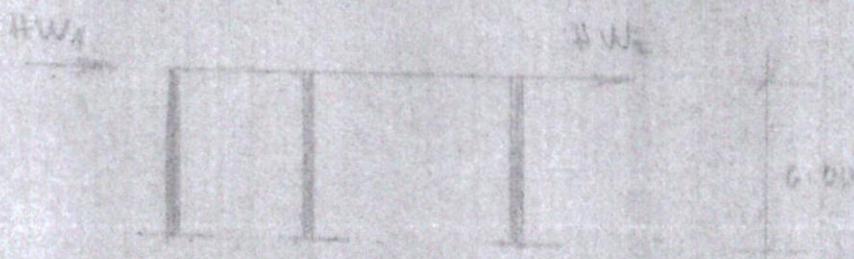
$$U_1 = 0,040 \times 6,00 \times 0,8 = 0,192 \text{ t/h}$$

$$W_1 = 0,040 \times 6,00 \times 0,4 = 0,120 \text{ t/h}$$

Výkon je závislý na počtu jednotek užití, A

To znamená, maximální výkon je určen jednotkou

SAC 6 M



Wert für ΔV

$$\Delta V_1 = \frac{1}{2} \times 12.0 \times 0.432 + \frac{1}{2} \times 2.5 \times 0.216 \times 2$$

$$\Delta V_1 = 2.80 + 0.54 = 3.34 \text{ t}$$

$$H_{\text{eff}} = \frac{1}{2} \times 6.00 \times 0.216 = 0.36 \text{ t}$$

$$\Delta H = 3.34 - 0.36 = 2.98 \text{ t}$$

Fließzeit von 100000 t ist 100000

$$H_{\text{eff}} = 4.00 \times 6.00 = 24.00 \text{ t}$$

Fließzeit von 100000 t ist $100000 / 24.00 = 1 \frac{1}{2}$ Jahre

3. "

$$H_{\text{eff}} = H_B = \frac{24.0}{2} = 12.0 \text{ t}$$

Die 2 Strukturelemente haben bei 100000 t
Kreisfläche auf der einen Seite unter einer d²

$$\text{Pis } H_{\text{eff}} = d^2 = 4.0^2 = 16 \text{ dm}^2$$

$$\text{Pis } H_{\text{eff}} = d^2 = 4.0^2 = 16 \text{ dm}^2$$

$$\text{Pis } H_{\text{eff}} = d^2 = 1.0^2 = 1 \text{ dm}^2$$

$$2 \cdot 16 = 471 \text{ dm}^3$$

Pis 100000 t bei 100000 t

Pis H_{eff} , Pis H_{eff}

$$H = 12.0 \times \frac{0.4}{4.71} = 1.02 \text{ t}$$

Pis H_{eff}

$$H = 12.0 \times \frac{0.4}{4.71} = 1.02 \text{ t}$$

Wert für ΔV bei 100000 t ist 100000 t

$$P_{SR} = \frac{13,67 + 3,854}{2} = 8,762 \text{ t}$$

$$M = \pm 1,63 \text{ t/m}$$

$$e = \frac{1,63}{8,762} = 0,060 \text{ m}$$

HALB SEKSEKTOR

$$F = 40 \times 40 = 1600 \text{ cm}^2$$

$$W = \frac{40 \times 40^2}{6} = 10700 \text{ cm}^3$$

$$\delta_{1/2} = \frac{24600}{1600} = 15,375 \text{ cm}$$

$$\delta_{1/2} = 15,375 \pm 15,3 = \left\{ \begin{array}{l} + 30,7 \\ - 6,1 \end{array} \right\} \text{ cm}$$

Pos 4/12

$$P_{SR} = \frac{18,17 + 43,54}{2} = 30,855 \text{ t}$$

$$M = \pm 1,63 \text{ t/m}$$

$$\delta_{1/2} = \frac{30855}{1600} = 19,281 \text{ cm}$$

$$\delta_{1/2} = 19,281 \pm 15,3 = \left\{ \begin{array}{l} + 34,6 \\ - 4,0 \end{array} \right\} \text{ cm}$$

Pos 4/15

$$P_{SR} = 59,386 \text{ t}$$

$$M = \pm 8,64 \text{ t/m}$$

$$e = \frac{8,64}{59,386} = 0,145 \text{ m}$$

HALB SEKSEKTOR

$$V = \pi r^2 h = 3,14 \cdot 200 \text{ cm}^3$$

$$W = \rho V g = 30600 \text{ cm}^3$$

$$\sigma_{1/2} = \frac{59380}{2800} = \frac{864000}{32600}$$

$$\sigma_{1/2} = 21,2 \pm 27,2 = \left\{ \begin{array}{l} +47,4 \\ -6,0 \end{array} \right\} \text{ kg/cm}^2$$

OBDIZOMINATO DO SU STUBOVI POS H/13.

Pos H/14 DATI ARHITEKTUROM, A IZDJE PAUČAKU
UOKUGNU PODNUCU - PODUŽNOM TO SU KAPONI
ZADNOJEVANACI A KAO CIKLOD STUBA

Pos H/15 ODE IDU OKO SO kg/cm² TE SE DI-
MENZIJE STUBOVA ZADRŽAVAJU VAO JE TO
VEĆ PRORACUNATO.

ONDE SE DAJE ZOŠ I SLEDEĆA KONTROLA:
DODATKI MOMENAT USLED DELIMICNOG UVEĆ
STUPA Pos H/5 i Pos H/6 iznose:

ZA STUB Pos H/13 od Pos H/15

$$\Delta M = 4,56 \times \frac{800^2}{40} = 7,35 \text{ kN}$$

ZA STUB Pos H/14

$$\Delta M = 4,56 \times \left(\frac{8100^2}{40} - \frac{3,50^2}{40} \right) = 5,92 \text{ kN}$$

ZA STUB Pos H/15

$$\Delta M = 4,56 \times \frac{3,50^2}{40} = 1,43 \text{ kN}$$

KONTROLA KAPONA:

Pos H/13 $P = 24,60 \text{ kN}$

$$M = NW + \Delta M = 1,63 + 7,35 = 8,98 \text{ kN}$$

$$e = \frac{24,60}{8,98} = 0,27 \text{ m}$$

VELIKI EX SCENTRATOR

$$M_a = 35,8 \left(0,27 + \frac{0,37 - 0,03}{2} \right) = 10,30 \text{ t}$$

$$A = \frac{37}{\sqrt{\frac{1030000}{40}}} = \frac{37}{161} = 0,230$$

$$\sigma_b/a \sim 105/1400 \text{ kg/cm}^2$$

$$T_a = \frac{10,30}{140 \times 0,9 \times 0,37} = \frac{24,60}{1,40}$$

$$T_a = 21,8 - 17,2 = 4,6 \text{ cm}^2$$

$$Postoji 2/22 \rightarrow T_a = 7,60 \text{ cm}^2$$

ZADOVOLJAVI I NEISPODICEVU UZPOU
UBECIOTU DO GRANICE.

Pos 4/14

$$P = 35,8 \text{ t}$$

$$M = M_w + \Delta M = 1,63 + 5,92 = 7,55 \text{ t}$$

$$e = \frac{7,55}{35,85} = 0,21 \text{ m}$$

$$M_a = 35,8 \left| 0,21 + \frac{0,37 - 0,03}{2} \right| = 13,30 \text{ t}$$

$$A = \frac{37}{\sqrt{\frac{1330000}{40}}} = \frac{37}{133} = 0,209$$

$$\sigma_b/a \sim 105/1400 \text{ kg/cm}^2$$

$$T_a = \frac{13,30}{140 \times 0,9 \times 0,37} = \frac{35,86}{1,40}$$

$$T_a = 28,8 - 25,4 = 3,4 \text{ cm}^2$$

$$Postoji 2/22 \rightarrow T_a = 7,60 \text{ cm}^2$$

ZADOVOLJAVI I NEISPODICEVU UZPOU
UBECIOTU DO GRANICE

Pos 4/15 P = 59,35 t

$$M = M_w + \Delta M = 2,64 + 6,43 = 10,07 \text{ t}$$

$$G_{\text{vpl}} = 21,9 + 30,8 = \left\{ \begin{array}{l} \pm 52,6 \\ \pm 9,6 \end{array} \right\} \text{N/cm}^2$$

$$G_{\text{vpl}} = 21,9 + 30,8 = \left\{ \begin{array}{l} \pm 52,6 \\ \pm 9,6 \end{array} \right\} \text{N/cm}^2$$

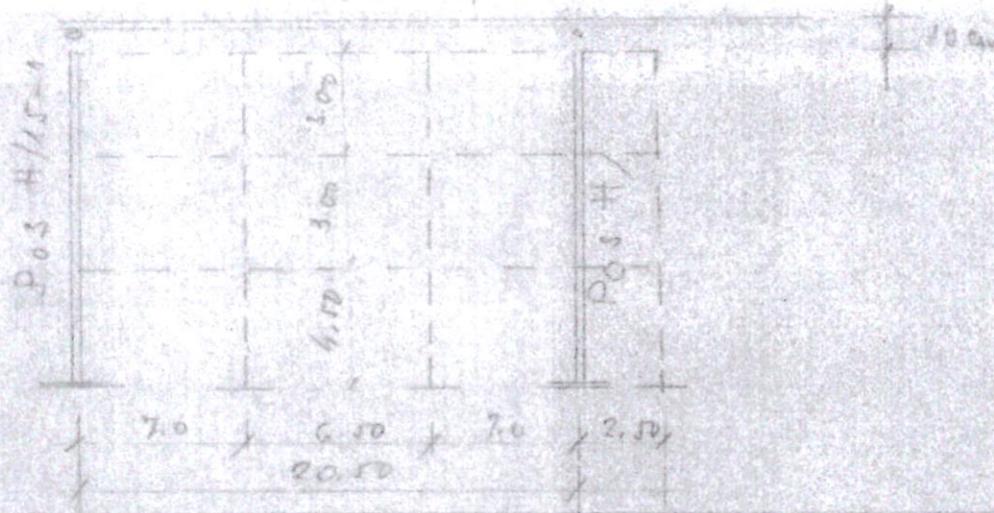
i odtocne presne zadovoljuva.

- PRORAČUN KALKULUA -

Pos. H/16 - KALKULUS: KOŠTICU

DISPONICIJONA ŠEMA

POS. H/1



a. Pos. H/16-1 senčilož

KONSTRUKTIVNO 30/40

ADMATURA 2 x 30x14

UZDUŽNIJE 4/6/20 cm

b. Pos. H/16-2 Podulava

$$l_{\text{cp}} = 7,00 \text{ m}$$

$$q = \frac{210 \cdot 0,38 \times 2,70 \times 1600}{\text{Podulava } 0,40 \times 5,00 \times 2500} = 1,65 \text{ t/m}$$

$$M_u = \pm 2,05 \times \frac{7,00^2}{8} = \pm 10,5 \text{ t.m}$$

$$D_1 = 2,05 \times \frac{7,00}{2} = 7,00 \text{ t}$$

$$D_2 = 2 \times 7,20 = 14,40 \text{ t}$$

$$T_a = \frac{10,0}{14 \times 0,9 \times 0,37} = 20,2 \text{ cm}^2$$

U Polju, poviti 1/2 od l/5 raspona.
HAD OSLOHCU DODATI 1/5 ARMATURE
UZEGIJE 1/8/130 + 13, RASPODJALE 15cm
HA 8/5 OD OSLOHCA

RASPOD l = 2,5 m ARMIRATA T_a = 10,0 cm²

c. Pos H/16-3 - STUB

$$l_K = 13,00 \text{ m}$$

OPTERECENJE "

1. OD SERVILAZA	$0,40 \times 7,00$	= 2,80
2. OD PODVLAKA	$2 \times 14,40$	= 28,80
3. STUB	$0,40 \times 0,40 \times 2,5 \times 13,00$	= 5,20

Podesak armature uko Pos H/13

$$\sigma = \frac{36800}{1752} = 21 \text{ kg/cm}^2$$

OBZIDOM HA VELIKU VITKOST

$$l_K = \frac{1300}{40} = 33 < 35$$

HA POUZDANOJ UZIČAJU "

- 12 meseci servilaza Pos H/16-1 i Pos H/11
ostaviti dilataciju 10 cm

— PREDHODA TAKADA —

Pos H/18 - HA STREŠNIČA

MONTAŽNI POSATI

$$l_0 = 5,60 \text{ m}$$

$$t = 6,00 \text{ m}$$

Povrchové vložky do sálu



$$T = 4.5 \times 3 = 2 \times 3.0 \times 2.5 = 33 \text{ m}^2$$

$$g_0 = 0.0522 \times 2100 = 132 \text{ kg/m}^3$$

OPTICKÉ OSY

$$1. \text{ V obouzadlech } 120 \cdot 60 \times 0.35 = 0.1 \text{ kg/m}^3$$

$$2. \text{ V osách } -132 \text{ kg/m}^3$$

$$3. \text{ V jedné z } 60 \times 0.35 = 21 \text{ kg/m}^3$$

$$g = 174 \text{ kg/m}^3$$

$$4. \text{ SNEH-VETR } 3 = 35 \text{ kg/m}^3$$

$$g = 209 \text{ kg/m}^3$$

$$M = 209 \times \frac{6.00}{8}^2 = 945 \text{ kg/m}$$

$$F_a = \frac{945}{1400 \times 0.9 \times 0.42} = 1.80 \text{ cm}^2$$

$$\text{VLOŽKA DO } 3 \# 12 \text{ SA } F_a = 3.39 \text{ cm}^2$$

$$\text{VOLN. } 2 \# 8 \text{ SA } F_a = 1.57 \text{ cm}^2$$

$$\text{VZDHNICE } \# 6/20 \text{ cm}$$

KRATKE VLOŽKY SÚ ISOM AOM 12000 TIT.
POČSOKA "E"

OCASOVÉ

$$Dg = 209 \times \frac{6.00}{2} = 627 \text{ kg}$$

$$Rg = 174 \times \frac{6.00}{2} = 522 \text{ kg}$$

$$Ds = 35 \times \frac{6.00}{2} = 105 \text{ kg}$$

-> A -

• 7.19 = Ustojce TE na krovku des zloženého CPu vzd.

$$l_0 = 3,05 \text{ m} ; l = 1,05 + 3,05 = 4,10 \text{ m}$$

$$a_0 = 1,75 \text{ m} ; \alpha = 1,05 + 1,75 = 2,80 \text{ m}$$

OPTICKÉ ČÍČKO

$$1.00 \text{ Pos } H/18$$

$$g_1 = \frac{E_2}{0,35} \times 2 = 5000 \text{ kg/m}$$

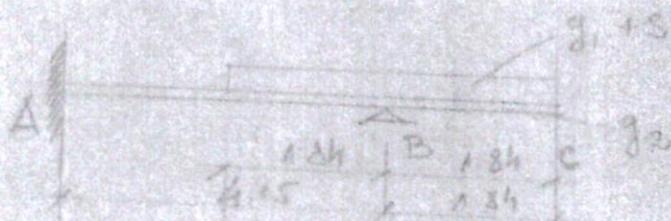
$$2.00 \text{ Pos } H/18$$

$$S = \frac{105}{0,35} \times 2 = 600 \text{ m}$$

SUPSTAVENO

$$g_0 = 0,00001 \times 9100 = 0,091 \text{ kg/m}$$

SISTEM PODĚLÍ



ÚTÍKAČI POD VALNOU OPTICKÉ ČÍČKO

$$M_B = -(3,00 + 0,43) \times \frac{1,80^2}{2} = -6,00 \text{ m}$$

$$Bd = (3,00 + 0,43) \times 1,80 = 6,35 \text{ t}$$

Počtože je základní, tože

$$+ \frac{2Ma}{3} = \frac{g_2 l^3}{24} + g_1 l^3 \left(\frac{1}{3^2} + \frac{1}{288} \right) - \frac{M_B l}{3}$$

$$Ma = +1,50 \left[\frac{g_2 l^3}{24} + g_1 l^3 \left(\frac{1}{3^2} + \frac{1}{288} \right) - \frac{M_B l}{3} \right]$$

$$Ma = +1,50 \left[0,091 \times \frac{4,10^3}{24} + 3,00 \times 4,10^3 \left(\frac{1}{22} + \frac{1}{288} \right) - \frac{6,00 \cdot 4,10}{3} \right]$$

$$Ma = 1,50 [0,32 + 0,18 - 2,00] = -2,25 \text{ m}$$

SPODNÍ ALEBO VYŠE

$$\Delta_1 = -B_S = -\frac{6,00 + 2,25}{4,10} = -1,976$$

6.15

$$B_0 = \frac{1}{2} \times 0.60 \times \frac{1}{2} + \frac{0.02 \times 0.5 \times 0.23}{0.15} = 5.23t$$

$$\Delta d = 2.15 - 1.97 = 0.18t$$

$$\Delta t = -(5.23 + 1.97) = -7.20t$$

$$B_{dt} = +0.35t$$

Reactions

$$\Delta = 5.15t$$

$$S = 7.20 + 6.35 = 13.55t$$

Utriculus oblongus

$$B_{dt} = -0.60 \times \frac{1.84}{2} = -1.02t$$

$$B_{dt} = 0.60 \times 1.84 = +1.02t$$

$$\frac{2.15t}{3} = +0.17 \left(\frac{1}{32} + \frac{1}{288} \right) = \frac{1.02t}{3}$$

$$\Delta d = 1.10 \left[0.60 \times 1.84 \left(\frac{1}{32} + \frac{1}{288} \right) - \frac{1.02}{3} \right]$$

$$\Delta_d = 1.10 (0.94 - 0.34) = -0.45t$$

Sphaeralcea glauca

$$\Delta_S = -0.35 = -\frac{1.02 \times 0.25}{0.15} = -0.36t$$

$$\Delta_d = 0.60 \times 1.84 \times 0.25 = 0.74t$$

$$B_{dt} = \frac{0.60 \times 1.84 \times 0.25}{0.15} = 0.86t$$

$$\Delta d = 0.74 - 0.36 = 0.38t$$

$$\Delta d = -(0.36 - 0.36) = -1.22t$$

total = -0.14t

$F_{a1} = 5,82 \text{ t}$

$$F_{a2} = 1,22 + 1,10 = 2,32 \text{ t}$$

WYSIĘGŁOŚĆ DŁUŻEŃ W DLA MIESZKANIA I KOMINÓW

$$M_{B1} = 6,00 - 1,02 = 7,02 \text{ t}$$

$$M_{B2} = 6,35 + 1,10 = 7,45 \text{ t}$$

$$\frac{\sqrt{2} \cdot 42}{\sqrt{702,000}} = \frac{42}{133} = 0,315$$

40

50% < 10% μ/m^2

$$C = \frac{7450}{40 \times 0,9 \times 12} = 4,90 \text{ kg/m}^2 < 7,50 \text{ kg/m}^2$$

$$F_a = \frac{702}{14 \times 0,9 \times 0,02} = 13,30 \text{ m}^2$$

WYSIĘGŁOŚĆ $q_1 / 20 \text{ cm}$

WYSIĘGŁOŚĆ

$$\Delta d = A_s = 0,16 - 0,12 = 0,04 \text{ t}$$

$$Bl = -(7,20 + 1,22) = -9,42 \text{ t}$$

$$M_A = -(2,25 + 0,25) = -2,50 \text{ t}$$

$$NB = -(6,00 - 1,02) = -7,02 \text{ t}$$

12. DIAGRAMA (3) (Ad 1.5c) WYSIĘGŁOŚĆ DŁUŻEŃ

CELĂ GRADU PREDSTAVĂ UZINA TIPICĂ DE PROIECTARE

ZYGOCĂSĂU 1971. UZINA TIPICĂ DE PROIECTARE

ARHITECTURĂ 70% MECANICHE PASCA

$$\text{ZYGOCĂSĂU } Ad_1 + F_a = 9,40 \text{ t}$$

$$\text{MECANICHE } Ad_1 - F_a = 13,30 \text{ t}$$

WYSIĘGŁOŚĆ $q_1 / 20 \text{ cm}$

$$C_{Bl} = \frac{9,42}{6 \times 0,9 \times 10,12} = 0,30 \text{ kg/m}^2 < 7,50 \text{ kg/m}^2$$



$$I = 100 \times 60 \cdot 24 \times 29 = 3064 \text{ cm}^4$$

$$J = \frac{1}{12} \times 100 \times 45^3 + 100 \times 29 \times 100^2 = 1000000 \text{ cm}^4$$

$$g_N = 0.064 \times 80 = 5.12 \text{ t/m}$$

OPREZNIČKI ČINIG —

A. STATIKI

1. nosilj	12 t	0,6
2. nosilj		0,32
3. nosilj		0,46

$$q = 0.64 \text{ t/m}$$

4.00 POS A/B

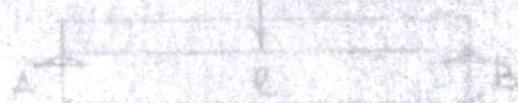
$$R = 42,44 \text{ t}$$

B. SNEG + VETAR

$$\Gamma \text{ JEDNANJE PODLEŽE} \quad c = 0,36 \text{ t/m}$$

$$6.00 \text{ POS A/B} \quad c = 2,32 \text{ t}$$

— STATIČKI SISTEM —



DEFINICIJE UKLJUČEWA GRADA (ELASTICNO
UKLJUČEWA)

$$V_{18} = \frac{9\pi}{5} \times \frac{3}{2}$$

$$H_1 + H_2 = -M_{20} = -0,57 \times \frac{6,0^2}{30} = -12,52 \times \frac{6,0}{5}$$

$$H_1 + H_2 = -2,50 = -16,20 \text{ m}$$

$$H_{AB} = +18,57 \text{ m}$$

$$4d \cdot 0,6 = 0,64 \times \frac{6,0}{2} + \frac{12,52}{5} = 6,70 \text{ m}$$

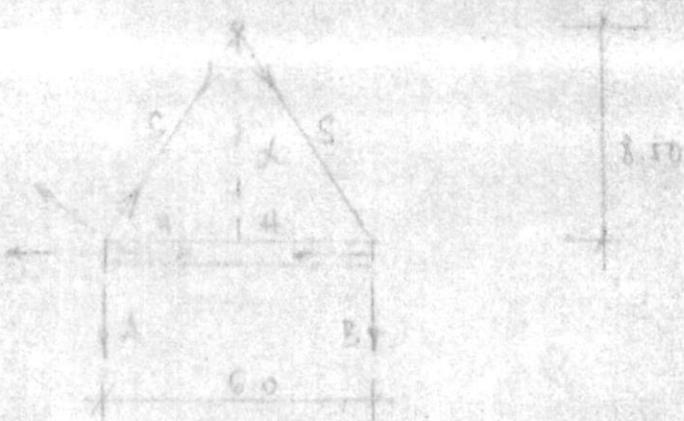
- UNEAKSI SU SHEGLI -

$$H_1 + H_2 = -2,50 \times \frac{6,0^2}{30} = -2,50 \times \frac{6,0}{5} = -3,00 \text{ m}$$

$$H_{AB} = +3,14 \text{ m}$$

$$4d \cdot 0,6 = 0,64 \times \frac{6,0}{2} + \frac{9,12}{5} = 7,46 \text{ m}$$

UNEAKSI OS VELIKYIE (SISTEMA POKRICH)



$$\operatorname{tg} \alpha = \frac{6,00}{3,10} = 0,333$$

$$\operatorname{tg} \alpha = \frac{0,333}{\sqrt{1+0,333^2}} = 0,333$$

$$\operatorname{tg} \alpha = \frac{1}{\sqrt{1+0,333^2}} = 0,942$$

$$M_1 = 3.5 \times (1.82 + 0.1 \times 0.2) = 2.7$$

$$q = \frac{4a}{\sqrt{2a+e}} = \frac{4a}{\sqrt{a^2+9}} = 6,247$$

10163 15,2 kg/m²
16 x 0,9 x 42

$$S \leq \left(\frac{g}{30h} + q \cdot l^2 + \frac{l}{48} \cdot Ql^2 \right) : \frac{A}{30}$$

$$q = 0,64 + 0,10 = 0,74 \text{ t/m}^2$$

$$Q = 15,55 + 2,30 = 15,85$$

$$S \leq \left(\frac{5}{30h} \times 7,40 \text{ t/m}^2 \times 6,0 \times 10^{-3} \text{ m}^2 \right)$$

$$+ \frac{1}{48} \times 15,85 \text{ t/m}^2 \times 6,0 \times 10^{-3} \text{ m}^2 \times \frac{1}{2,0 \times 7,89 \times 10^{-3} \text{ m}} \text{ t/m}$$

$$S \leq \left(\frac{5 \times 7,4 \times 1,296 \times 10^{-4}}{3,8h \times 10^2} + \frac{1,587 \times 0,789 \times 10^{-12}}{4,8 \times 10^2} \right) \text{ t/m}$$

$$\times \frac{1}{2,0 \times 7,89 \times 10^{-3} \text{ m}}$$

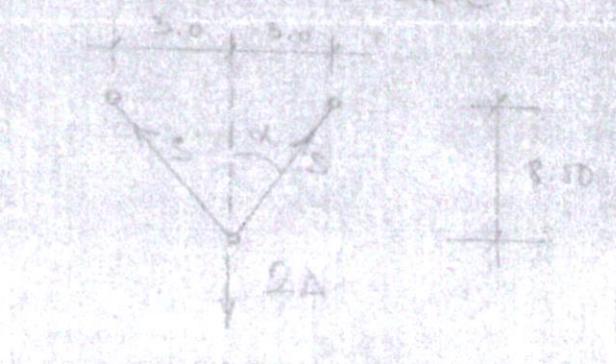
$$S \leq (1,24 \times 10^{-10} + 7,20 \times 10^{-10}) \times \frac{1}{4,8 \times 10^{-3}}$$

$$f \leq \frac{s \cdot h}{11,80} = 0,15 \text{ cm} \approx \frac{1}{1200}$$

Kako je ovaj učinjeno računat rezultujuća uticaja
neke ulične stene na horizontalne primadice
sile koji ga snažnije visine od $\frac{1}{2}$ tog učinka
treba računati sa svim uticajem na deo stene
OPLATU OSTAVITI NEGRADJENU

Poz A/21 - Nečlanjiva

SISTEM PO SKICI



$A = 12.5 \text{ cm}^2$

W BETONU VD $G_b = 120 \text{ kg/cm}^2$

$$M = 126 \times 120 \times 244 = 36$$

UDUNGBBO U-SVALBARD-1968

4 - 7.2 t

↳ [Mueller se traspolaza sas Efectos](#)

7. ^W_m doublets of second kind

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$$G = \frac{A + B \rho}{C + D \rho} = 6250 \text{ kg/m}^3$$

III. *Macrolides* (continued)

$$W = \frac{140}{62.5} = 2.2$$

REVIEWS 145

1981-667 5-140-160

- Dachfläche

- Dachfläche 45/120

- Dachfläche 30%

- Flanschfläche 3,60 x 5,0 = 18,00 m²

- Flanschfläche 0,48 x 1,20 x 2,50 = 1,44 m²

- Flanschfläche 3,30 x 6,0 = 19,80 m²

1000 kg/m³

- Mischung

$$\text{Mischung} = \frac{1,473 + 1,473}{2} = 1,473 \text{ m}^3$$

- Dachfläche

$$M = 1,473 \cdot \frac{6,00^2}{4,00} = 7,40 \text{ m}^3$$

- Dachfläche

$$S = 1,473 + \frac{6,00^2}{2} = 9,90 \text{ m}^2$$

$$P_1 = 2 \times 9,90 = 19,80 \text{ t}$$

- Konkurrenz im Polen

$$T_{\alpha} = \frac{1,40}{1,00 \times 0,9 \times 0,42} = 14,00 \text{ m}^2$$

$$7,40 \text{ m}^3 / T_{\alpha} = 14,00 \text{ m}^2$$

$$\text{Pol. doppelt } \Rightarrow T_{\alpha} = 8,00 \text{ m}^2$$

$$\text{Pol. doppelt } \Rightarrow T_{\alpha} = 3,00 \text{ m}^2$$

- Ausnutzung wird verringert

$$T_{\alpha} = \frac{7,40}{1,00 \times 0,9 \times 0,42} = 19,40 \text{ m}^2$$

$$\text{Pol. (doppelt) doppelt } \Rightarrow T_{\alpha} = 16,00 \text{ m}^2$$

$$\text{Pol. (doppelt) doppelt } \Rightarrow T_{\alpha} = 3,00 \text{ m}^2$$

$$T_{\alpha} = 1,50 \text{ m}^2$$

- Ausnutzung 8/20

Environ Biol Fish (2007) 80:33–40
DOI 10.1007/s10641-007-9300-0

$$D W_4 = \frac{1}{2} \times 5 \times 4 = 10 \text{ cm}$$

$$M_{\text{BH}} = 1.67 \times \frac{1.76}{5} + 1.40 \times 10^{10}$$

3.34 m.s⁻¹, 25.6 m.

$$W = 40 \times 85^{\circ} = 3470 \text{ sec. min.}$$

$$\text{S}_{1/2} = \frac{91450}{3490} = 26500$$

$$(V_{\text{th}} + 2)/5 \cdot 0.2 = \frac{1}{4} \cdot \frac{-4.9}{5} \cdot \log(\alpha_0^2)$$

b. Pos 1/23-2 See Dated 5a u.s.

1000 Dec. 14, 1945

9-16-1964 100% / 100% 100% / 100%

2010-11/12 - 100, 80 mm

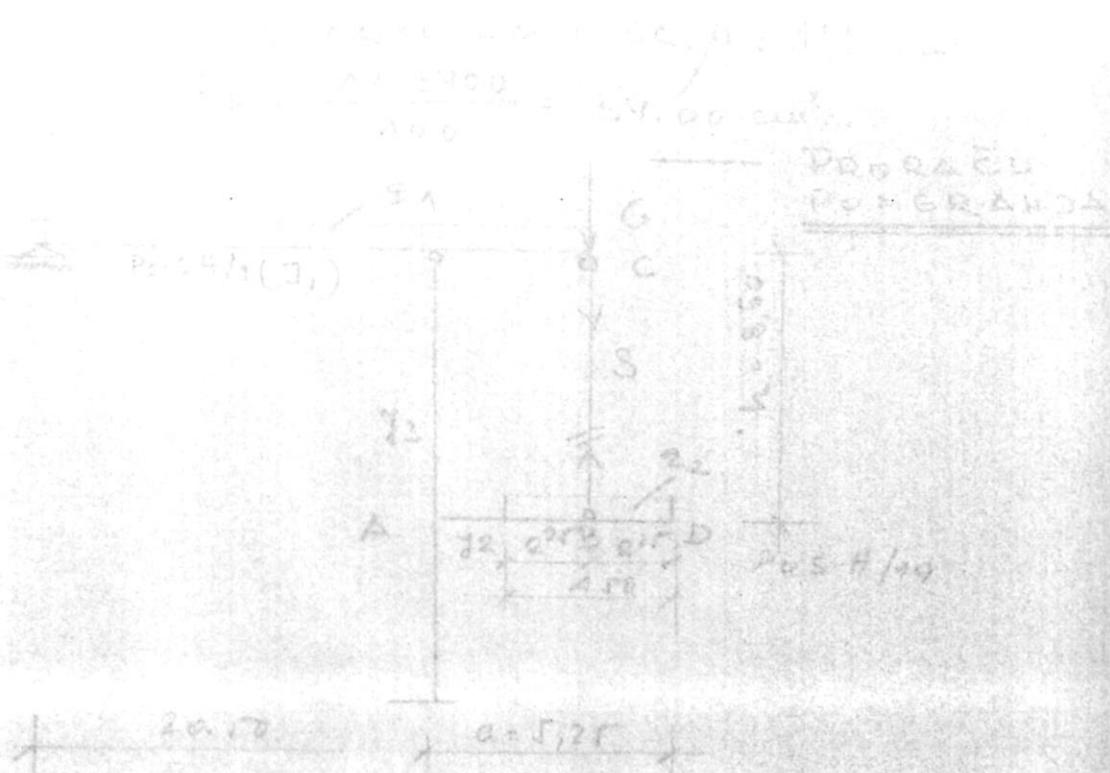
- 40 -

卷之三十一
197. 天象大

$M_{\text{H}_2} = 2 \times 10^{-6} \text{ g cm}^{-2} \text{ s}^{-1}$

$\frac{5}{2} \times \frac{1000000}{1000000} = \frac{5000000}{1000000}$

$$S_{16} = S_2 \pm 4.4 = \frac{S_2 - 7.6}{4.2} \text{ mg/mmol}$$



PRI PRORACUNU POS 4/19 UVIDJE SE DA JE
MONIHAT NA HALI TAKO DA MOZE UZETI DA
GREDA S PREPUTOM A B D BUJO UNIKESTENA U

POSEČANJEN ZATEGE DOVODJANO SLEDEĆE
IZ USLOVA:

$$\delta_c = \delta_e$$

ZAGREBU POS 4/1 06/06

$$q_1 = 3,83 \text{ t/m}$$

$$G = 1,30 \text{ t/m}$$

$$y = 9,2078 \text{ m}^3$$

ZAGREBU POS 4/19 06/06

$$(Q = q_2 \times 4,56)$$

$$Q = 20,32 \text{ t} \quad (\text{VIDI POS 4/21})$$

$$J = \frac{0,40 \times 0,45^3}{12} = 0,0030 \text{ m}^4$$

ZAGREBU POS 4/21 06/06

$$T = 2 \times 0,0030 = 0,0060 \text{ m}^2$$

E - MODUL GLASTICNOSTI UZETI 26 GIGAPASCAL

$$\left(\frac{1}{\lambda^2} + \frac{\beta_2}{\lambda^2} - \frac{6\gamma_2}{(\lambda^2 - 3)} \right) = \frac{6\alpha^2}{\lambda^2 - 72}$$

$$\left(1 + \frac{J_2}{J_1}\right) = 2.07$$

$$\frac{I_2}{I_1} = \frac{30}{20\%} = 150$$

$$\frac{1.72}{2.07} = \frac{8.5 \times 0.0030}{8.25 \times 0.0244} = \frac{8.5}{8.25} = 0.0675$$

$$S = \frac{Q}{140,000 \pi - 0.0076}$$

- DANGI SLUČAJ -

Sue G- up Post 4/1

$$\delta r = \frac{Sa^5}{3EJ_1} + \frac{4a^3}{l}$$

$$S_0 = \frac{Qa^2}{332} - \frac{Sa}{372}$$

ZAHORACHE JUDASHEZ DE HOGAR MATE VALA VD
DE QOR LORA VTO LE VUL Y DONG LURCADO

58-5b

$$\frac{G}{J_2} \left(1 + \frac{J_2}{J_1} \right) = \frac{J_2^3}{33 J_2} \left(1 - \frac{4 J_1}{8 J_2} \frac{J_2}{J_1} \right)$$

$$J_2 = 0.72 \quad \begin{matrix} 10 \\ 10 \\ 10 \\ 10 \\ 10 \end{matrix}$$

$$J_1 = 0.72 \quad \text{t/m}$$

$$\frac{G}{J_2} = \frac{2497.125 \times 3}{1220.32 \times 2072} = 0.00023$$

$$\frac{G}{J_1} = 0.00023$$

$$S = G = \frac{A \cdot 0.00023}{A + 0.00023} \approx 0.23$$

Pracovní páky mohou být poč. 1/19

Poč. 1/19 - Příprava na výrobu měřicích

prvků pro měření tlaku vody v dle požadovaného

#20. Dátka o měření tlaku vody v dle požadovaného
počtu pánky.

MATERIÁL A METODICKÝ POKRÖVÍ

Poč. 1/19 - Měřicí jednotky

100 cm

1 - Poč. 1/19 - Poč. 2/19

Co je potřeba:

1 - Poč. 1/19 - Poč. 2/19 - Poč. 3/19 - Poč. 4/19

1 - Poč. 1/19 - Poč. 2/19 - Poč. 3/19 - Poč. 4/19

1 - Poč. 1/19 - Poč. 2/19 - Poč. 3/19 - Poč. 4/19

1 - Poč. 1/19 - Poč. 2/19

1. $\frac{1}{2} \times 10^6$ m^3 \times 10^3 kg/m^3 \times 10^3 J/kg \times 10^{-3} GJ

$\frac{1}{2} \times 10^6 \times 10^3 \times 10^3 \times 10^3 \times 10^{-3} = 5 \times 10^{10} \text{ GJ}$

$\frac{1}{2} \times 10^6 \times 10^3 \times 10^3 \times 10^3 \times 10^{-3} = 5 \times 10^{10} \text{ GJ}$

$1600 \times 0.4 \times 0.75 = 4800 \text{ m}^3$

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$$G = 0.4382 \cdot 70 \times 2.40 = 772.0 \text{ Nm}$$

Long Island University

4-115 SA ONE STRANGE

Kazbecas

of Chis. & the streng.

4/26 - ZATSGA -

Dōzen 20/2

KONSTRUKTIVE 4/12

— STEPEWISTE —

DIEGO ZICRA

$$D_A = 1.8^{\circ} \quad D_B = 1.58^{\circ}$$

$$D_C = 1.72^{\circ}$$

$1.8^{\circ} \times 1.58^{\circ} = 11.60^{\circ}$

OPERAÇÕES

1) TRAÇAR $\alpha_1 \beta_1 \gamma_1$

2) PROJETAR $\alpha_1 \beta_1 \gamma_1$

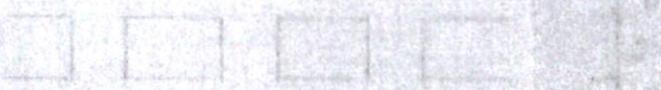
3) TRAÇAR

$\alpha_2 \beta_2 \gamma_2$

$\alpha_3 \beta_3 \gamma_3$

$\alpha_4 \beta_4 \gamma_4$

SILICA PHASE



$$10 \quad 40 \quad 10$$

$$+ 2 \quad + 2 \quad + 2$$

$$M = \frac{912}{16} = 57 \text{ g/mol}$$

$$D_A - D_B = 6.97 \times \frac{11.60}{2} = 39.81$$

$$q = \frac{37.0}{\sqrt{8300}} = \frac{37.0}{85.5} = 0.435$$

$D_A < 4000 \text{ mg/m}^3$

$$T_a = \frac{37.0}{1.60 \times 0.435} = 51.60 \text{ cm/m}$$

(ARMATURA SISTEMA U-BE 100 mm)

-8-1

Aug 13 1976 - 2000 ft.

Wetland

2000 ft.

1200 m 8/24/76

Wetland

2000 ft. 1000 m

D. 10/10 - Scutellaria (2nd count)

1200 m 8/24/76

Wetland

1000 ft.

2000 ft.

1000 m

Scutellaria

1000 m

1000 m

1000 m

D. 10/10 - 2000 ft.

q

st
100000

q
100

100

Par.

21.80

15.00

1000000000

May 40% no 1000000000

1. 1. 1962 - 1. 7. 1962, 8 km.
1. 1. 1962 - 31. 12. 1962, 8 km.

— Kontrolla Potporneho zidu —

— Kontrolla Potporneho zidu —
15/10/62 10:00 AM HBH 50

— Kontrolla Potporneho zidu —

Pos 1/28 - Potporne zid

ONAD potporne zidu je vzdalenost
medzi dve vodivky ob stravy
186,1 cm. Czadu 134,0 cm.

— Upravte vodivky ob stravy —

+ — Aby si vodivky ob stravy

zadali 96,5 cm (zadu) a 134,0 cm (vodivky ob stravy).

G. — Gotuji 1. 10. 1962 10:00 AM

— Potporne zid —

3 = 13,8 cm / m 100 cm = 138 cm

G. 1. 1. 1962 10:00 AM

$$\text{M} = \frac{138}{100} = 1,38 \text{ m} \quad (\text{do Pos Alz, Psp})$$

$$M = \frac{138}{6,01 \times 100} = 0,0227 \text{ m} \quad \text{do Metr} = 294 \text{ cm}$$

— Potporne zid —

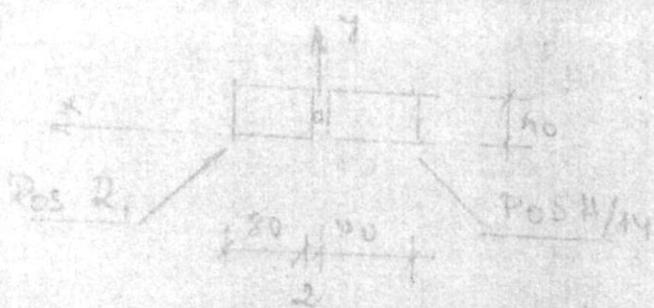
— Kontrolla Potporneho zidu —

— Kontrolla Potporneho zidu —

- Temelji -

Način řešení základů do dřevěných stojanů
včetně zavedeného uvedení
kánu do dosažení závěrečné (tu můžete)
objektu fundovaného na výšku po se vzdále
usuvat je dovoleno upřesnitelnou
zelení.

Pos H/T-1 - temelji - podle stan. Po. 1/1
- Diag 2/97A - 2000 -



UTICÁCÍ

Oo. Pos H/V

$$U = 35,90 \text{ t}$$

$$M_y = 13,30 \text{ t}$$

$$P_y = \frac{13,30}{8,50} = 2,40 \text{ t}$$

Oo. Pos R (Deo. "A")

$$U = 107,30 \text{ t}$$

$$M_x = 18,90 \text{ t m}$$

$$M_z = 9,70 \text{ t}$$

$$Z = G_0 = 15,94 / \text{m}^2$$

$$I_a = \frac{1,20 \cdot 35,00 + 169,80}{15,0} = 16,6 \text{ cm}^2$$

$$a = \sqrt{16,6} = 4,06 \text{ cm} \quad a \approx 4,00 \text{ cm}$$

$$\tilde{A}_a = 4,00 \times 4,00 = 16,0 \text{ cm}^2$$

$$W = 16,0 \times \frac{4,0}{6} = 10,7 \text{ m}^3$$

$$h = 2,00 \quad (111)$$

$$M_X = 13,30 + 2,00 \times 2,40 = 18,70 \text{ Nm}$$

$$M_Y = 18,70 + 2,00 \times 2,40 = 23,80 \text{ Nm}$$

$$G = 16,6 \times 2,0 \times 2,4 = 80,0 \text{ t}$$

$$V = 35,9 + 169 + 80,0 = 285,9$$

$$\tilde{G}_{1/2} = \frac{285,9}{16,6} \pm \frac{23,80}{10,7} \pm \frac{18,70}{16,6}$$

$$\tilde{G}_{1/2} = 18,0 \pm 2,40 \pm 1,80$$

$$\delta_1 = + 22,2 \text{ t/m}^2$$

$$\delta_2 = + 13,8 \text{ t/m}^2$$

Pos H/T 2 - Tonelj 18PUD 2500A Pos H/14



Utklasi 00 Pos H/14 18PUD

200 Pos H/14-1

+40

$$H = 35,90 \text{ t}$$

$$M = 13,30 \text{ Nm}$$

$$H = 2,40 \text{ t}$$

$$M \leq 18,70 \text{ Nm}$$

$$2A G_a = 15,0 \text{ t/m}^2$$

$$I = \frac{1,20 \times 35,9}{15,0} = 2,85 \text{ m}^2$$

$$a = 2,10 \text{ m} \quad T = 4,41 \text{ m}^2$$

$$W = 4,41 \times \frac{2,10}{6} = 1,51 \text{ m}^2$$

$$G = 4,41 \cdot 2,00 \times 2,4 = 21,4 \text{ t}$$

$$V = 35,9 + 21,4 = 57,2 \text{ t}$$

$$\sigma_{1/2} = \frac{57,20}{4,41} + \frac{18,10}{1,51}$$

$$\sigma_{1/2} = 13,0 \pm 11,6$$

$$\sigma_1 = +24,6 \text{ t/m}^2$$

$$\sigma_2 = +1,4 \text{ t/m}^2$$

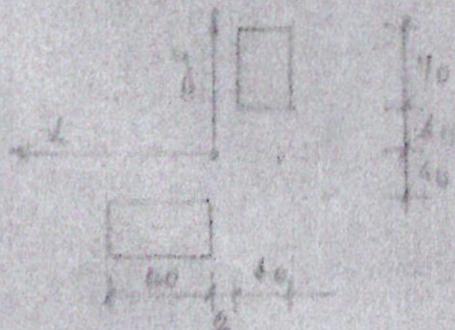
Pos H/1-3 venuelij isden pos w/H/4

UTICAS1 Pos H/4 471 CAD KOD Pos H/T-1

Grand Tenueng te miti tracutan windse
Dewasa prambanan kota C

Pos H/T-4 Touching ratio Pos H/15

Dit 2 Pos 2+3+3A



UTICAS1

CCP02 H/15

$$V = 5,7,10 \text{ t}$$

$$M = 2,60 \text{ m}^2$$

$$\sigma_1 = \frac{2,60}{3,15} = 0,80 \text{ t/m}^2$$

OD Pos. D₂

$$N = 122,90 \text{ t}$$

$$M_x = 10,90 \text{ t/m}$$

$$M_x = 5,60 \text{ t}$$

$$\Sigma A \bar{S} = 15,0 \text{ t/m}^2$$

$$F = \frac{A20((9,4 + 122,9))}{15,0} = 12,30 \text{ m}^2$$

$$a = 3,60 \text{ m}$$

$$F = 3,6 \times 3,6 = 12,96 \text{ m}^2$$

$$W = 12,96 \times \frac{3,6}{6} = 7,78 \text{ m}^3$$

$$R_n = 2,00 \text{ m}$$

$$My = 8,60 + 2,00 \times 1,60 = 10,80 \text{ t/m},$$

$$M_x = 10,90 + 2,00 \times 5,60 = 2,10 \text{ t/m}$$

$$G_x = 12,96 \times 2,00 + 2,00 = 62,2 \text{ t}$$

$$V = 59,4 + 122,9 + 62,2 = 244,5 \text{ t}$$

$$\delta_{N_2} = \frac{244,5}{12,96} - \frac{10,8}{7,78} + \frac{22,2}{7,78}$$

$$\delta_{N_2} = 18,7 \pm 1,5 \pm 2,7$$

$$\delta_1 = +23,1 \text{ t/m}^2$$

$$\delta_2 = +14,7 \text{ t/m}^2$$

Pos H/T-5 - Tenuelj ispod Pos H/A5

UTICAJI KAO KAO POS H/T-4

$$N = 59,4 \text{ t} \quad M_x = 8,60 \text{ t/m}$$

$$M_x = 1,60 \text{ t} \quad M_g = 14,80 \text{ t/m}$$

$$\Sigma A \bar{S}_0 = 15,0 \text{ t/m}^2$$

- 76 -

$$T = \frac{1,20 \times 59,4}{12,0} = 4,75 \text{ m}^2$$

$a = 2,10 \text{ m}$
 $b = 2,40 \text{ m}$

$$T = 2,10 \times 2,40 = 5,04 \text{ m}^2$$

$$W = 5,04 \times \frac{2,40}{6} = 2,02 \text{ m}^3$$

$$G = 5,04 \times 2,10 \times 2,4 = 24,2$$

$$V = 59,4 + 24,2 = 83,6 \text{ t}$$

$$\tilde{\sigma}_{1/2} = \frac{83,6}{5,04} \pm \frac{11,8}{2,02}$$

$$\tilde{\sigma}_{1/2} = 16,6 \pm 5,9$$

$$\tilde{\sigma}_1 = + 22,5 \text{ t/m}^2$$

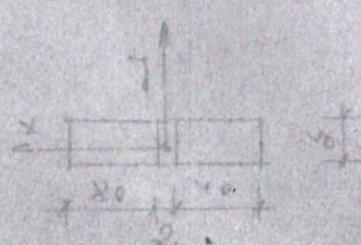
$$\tilde{\sigma}_2 = + 10,7 \text{ t/m}^2$$

Pos 4/T-6 - teoretički raspodjeljeni pos 4/T-5

UTICAJI POS 4/T-5 SIR KAO KOD POS 4/T-4

DUGOTRUDNIČKI BITI, SRACUNAT KAD SE
IZVRŠI POKRŠUJUĆI RJEŠENJE "C"

Pos 4/T-7 i T-8 teoretički raspodjeljeni pos 4/T-6



UTICAJI OD POS 4/T-6

$$U = 36,80 \text{ t}$$

$$M_x = 0 \text{ t.m}$$

$$M_y = 0 \text{ t.m}$$

UTICAJI OD POS R₂

$$U = 126,30 \text{ t}$$

$$M_x = 10,90 \text{ t.m}$$

$$M_y = 5,60 \text{ t.m}$$

Počítač užívá Pos. H/T-4

$$I = 12,96 \text{ m}^2$$

$$V_S = 7,78 \text{ m}^3$$

$$G = 62,2 \text{ t}$$

$$V = 36,80 + 420,30 + 62,20 = 425,3 \text{ L}$$

$$M_X = 10,46 + 2,00 \times 5,60 = 22,10 \text{ t/m}$$

$$\textcircled{C} \quad h_2 = \frac{225,3}{12,96} + \frac{22,10}{7,78}$$

$$\textcircled{C} \quad h_2 = 17,4 \pm 2,8$$

$$S_1 = + 20,2 \text{ t/m}^2$$

$$S_2 = + 14,6 \text{ t/m}^2$$

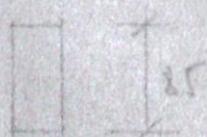
Pos. H/T-9 do Pos. H/T-11 tedy 3 stup. pos.

- H/MG

Užívání Pos. H/MG ještě vzdálenost Pos. H/T-7

Ověj. tedy bude vzdálenost užitá k výpočtu průřezu zde

Pos. H/T-12, T-13 tedy 2 stup. pos. Pos. H/T-3



$$H = 107,20 \text{ t}$$

$$M_g = 21,20 \text{ t/m}$$

$$\cancel{f_{40}} \quad 2 \Delta S_0 = 15,0 \text{ t/m}^2$$

$$I = \frac{12 \times 107,2}{15,0} = 8,6 \text{ m}^2$$

$$a = 2,1 \text{ m} \quad b = 4,3 \text{ m}$$

$$F = 0,4 \times 4,3 = 9,08 \text{ m}^2$$

$$W > 1,05 \times \frac{G_{1,2}}{6} = 6,50 \text{ m}^2$$

$$G_1 = 9,05 \times 2,0 \times 2,4 = 43,5 \text{ t}$$

$$G_2 = 10,5 \times 43,5 = 450,7 \text{ t}$$

$$\frac{G_1}{G_2} = \frac{43,5}{450,7} = \frac{21,2}{215}$$

$$Y_{1,2} = 16,7 \text{ t} = 3$$

$$T_1 = 4,90 \text{ t/m}^2$$

$$G_1 = 43,4 \text{ t/m}^2$$

10/2 - 49 - Tewerkturbo zirkonista

$$H = 5,60 \text{ t/m}$$

$$A = 8,50 \text{ m}^2$$

$$H = \frac{5,60 \text{ t/m}}{8,50 \text{ m}} = 0,66 \text{ t/m}$$



$$G = 2 \text{ m} \times 1,50 \times 2,10 = 6,30 \text{ t/m}^2$$

$$U = 5,60 + 9,00 = 14,60 \text{ t/m}^2$$

$$\mu_3 = 3,70 \times 1,50 = 5,55 \text{ t/m/m}$$

$$T_S = 4,00 \times 2,50 = 10,00 \text{ m}^2$$

$$W = 250 \times \frac{2,50}{6} = 1,04 \text{ m}^2$$

$$\tilde{\sigma}_{1/2} = \frac{14,60}{0,10} \pm \frac{5,50}{1,04}$$

$$\tilde{\sigma} = 15,84 \pm 5,40$$

$$\tilde{\sigma}_1 = 11,24 \text{ t/m}^2$$

$$\tilde{\sigma}_2 = 0,44 \text{ t/m}^2$$

GRALI TEHNIČKI ZA ZIDOVU
B/h = 50/70 cm

SVI TEMELJI HABITAT MB 16

Novi Sad, I. 1963.

PREPISAO:

Attegaj
(A. HERANDOŽIĆ)

RACUNAO:

3. Štamana Teflon
(ING. DRŠKO BERIĆ)
BR. OVLASĆENJA: 0

KONTROLISAO:

(ING. I. FADKAŠ)

NR: